

**REPORT
SOILS INVESTIGATION
PROPOSED BUILDING AT
800 KOST STREET
ALVIN, TX 77511**

**FOR:
COBALT ENGINEERING**

Lab. Job No.: 18-422

Report Number: 1812/1322



COASTAL TESTING LABORATORIES, INC.

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December 12, 2018

Cobalt Engineering
12005 Delany Road
La Marque, TX 77568

Reference: Soils Investigation

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Dear Mr. Hampton,

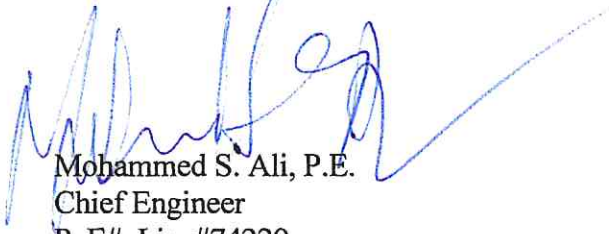
We are pleased to transmit the results of our soils investigation performed for the proposed building located at 800 Kost Street. Alvin, TX 77511.

The soils encountered in the exploration borings possess adequate strength to support the proposed structure.

In addition to our soils engineering capabilities, we can also provide complete concrete and asphalt testing and supervision services and Phase 1 & Phase 2 environmental studies. We would be pleased to work with you during the construction phase of this project.

We appreciate the opportunity to assist you in performing this study. If we can be of further assistance for this study or others, please contact us.

Yours very truly,
COASTAL TESTING LABORATORIES, INC.


Mohammed S. Ali, P.E.
Chief Engineer
P. E#. Lic. #74229

MSA/kd



PREFACE

The recommendations presented in this report were formulated on the basis of the soil conditions encountered in the exploration borings laboratory tests on soil samples obtained from these borings, engineering analysis and interpretation and judgement of the data derived.

Soil conditions other than those encountered in the exploration borings may exist at the site. Although our field technicians visually survey the site for surface expressions indicative of variable soil conditions (fill, borrow or drainage areas, faulting, etc.). Coastal Testing Laboratories, Inc. can not assume responsibility for the results of soil conditions inconsistent with those encountered.

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SCOPE AND PURPOSE

In this report we present the results of the soils investigation performed at the site of the proposed building. The purpose of this study was to evaluate the surface and sub-surface soils as related to the design of the foundation of the subject works.

SITE CONDITIONS

SURFACE CONDITIONS

The surface soils were relatively firm at the time of our investigation and presented no significant problems in moving the drill rig. However, under to inclement weather conditions, the surface soils are likely to become saturated and experience a reduction in shear strength. Therefore, to facilitate construction, the site should be shaped to provide adequate drainage.

SUB-SURFACE CONDITIONS

In accordance with the authorized scope of work, the sub-surface soil conditions were explored by drilling two (2) borings to a depth of 20' beneath the existing ground surface. The locations of the field explorations are shown on Plate 1. The Boring Logs are presented on Plates 2-3 and a detailed description of the soils is included.

The soils encountered in the borings are typical for this Deltaic Region. Generally, they are geologically recent surface soils composed of combinations of dark gray sand, silt and clay. These Recent soils are underlain by the older geologic formation. This formation typically is multi-colored and consists of silty and sandy clays, clays and clayey silts in color combinations of light gray, tan and red. These cohesive soils frequently are interbedded with layers and lenses of fine to medium sand and silty sand. Typically, these sand layers below a depth of 10-15 feet are saturated with water.

The older soils generally possess good to excellent strength properties but vary both horizontally and vertically and thus are not uniform. The compressibility characteristics are good, tending to be over consolidated in the upper portions and approaching normal consolidation with depth.

Sensitivity to moisture (expansive potential) of the surface and near surface soils was

evaluated by performing Atterberg Limits Tests on selected samples. The Plasticity Indices (PI) obtained for the soils in the top five feet are between 54 and 65. Any soil with a PI greater than 20 is considered expansive.

Ground water, if encountered, is shown on the boring logs; however, water levels will fluctuate seasonally.

DISCUSSIONS AND RECOMMENDATIONS

GENERAL REQUIREMENTS

We understand that the proposed building will be a one to two story structure and will impose light loads on the soils.

Utilizing the results of the laboratory tests, engineering analyses were performed. The results of these analyses and the observations of our field personnel form the basis of our recommendations.

Based on the soil conditions encountered in the borings, the site possesses suitable soils to support the proposed structure.

SITE PREPARATION

All surface vegetation should be stripped from the areas to be developed and wasted or stockpiled and used for "top dressing". If the exposed soils (at finished grade) are wet, they should be scarified and allowed to dry. The area should then be proof rolled with a rubber tired vehicle such as a partially loaded dump truck to detect any soft areas. Soft areas should be excavated and back-filled with compacted suitable materials compacted to approximate the surrounding soils. Holes resulting from excavation of stumps or other buried debris should be handled in the same manner. The subgrade should then be compacted to 95% of Standard proctor (ASTM D698).

STRUCTURE AND SLAB SUPPORT

Structure Support

Based on the soil conditions encountered in the borings, the structure can be supported

on drilled and belled piers.

Drilled and Belled Foundations

The structural loads may be supported on drilled and belled (underreamed) type footings. We recommend the piers be supported on the soils shown at a depth of 12' on top of very sandy clay layer which ever is higher. The soils at this depth should be stiff clays. Allowable soil bearing pressure for sizing pier bells is 3200 pounds per square foot. This pressure is for sustained loads and contains an applied safety factor of 3. It may be increased by 20 percent for short-term loads such as wind.

Ground water was encountered below the proposed foundation depth therefore, we do not anticipate that there will be a problem during drilling of the shafts. If ground water is encountered at that time the design engineer must be contacted immediately. Shafts should be poured as they are drilled and not be allowed to remain open for extended periods of time.

During drilling of the foundation piers, it is essential that the installation procedures be carefully inspected by an experienced person to assure that the bells are sized correctly and the bottoms are cleaned and on undisturbed soil. Failure to provide complete inspection services may result in improper foundations.

To minimize movement of grade beams as a result of soils expansion, the bottom of the beams are recommended to be isolated from the natural soils by at least 6 inch thick void boxes. A less effective alternate is isolation by 3.0' of non-expansive select fill having a Plasticity Index between 7 and 20. The structural engineers often question the merit of using void boxes; the decision to use them should be made by the design structural engineer/architect. Backfill against the outside face of grade beams is recommended to be select fill material.

It is our opinion that if founded in accordance with these recommendations, differential and total settlement will be negligible. Should soil conditions other than described herein be encountered, this office should be notified immediately.

Drilled Foundation Slab Support

The upper soils at this site are expansive. Consequently, they will exert non-uniform

uplift pressures and varying degrees of support for a slab placed upon them. These changes will relate directly to the changes in moisture content of the supporting soils. An increase in moisture results in an increase in volume and conversely, a decrease in moisture results in a shrinkage or decrease in volume. There is some evidence to indicate that the maximum change in volume occurs in seven-year cycles. We cannot confirm this theory, but we do know that several years can transpire before adverse effects from expansive soils become obvious.

There are several methods of handling expansive soils so that floor slabs are minimally affected. Best method is that the slabs be designed as structural slabs and isolated from the expansive soils by void boxes and supported by piers. A less effective alternate is to separate the slab from the natural soils by at least 4.0' of non expansive select fill.

Another alternative exists which is much less expensive and, we believe, provides an acceptable slab performance. The slab may be designed as a rigid slab-on-grade and not tied with the piers. It is intended that the slab be allowed to move vertically in response to volume changes of the soil. A post-tensioned slab may also be used in this manner.

EXPANSIVE CLAYS

The high plasticity clays at this site experience significant volume changes with changes in moisture content. During hot, dry periods the soil loses moisture and shrinks. Conversely, during extended wet weather cycles, the soil gains moisture and swells. This seasonal movement can exert considerable stresses on structures supported by these soils.

Under normal conditions, water evaporates from the surface of the soil and it replaced by water drawn by capillary action from below. When a floor slab and vapor barrier is placed on the surface, this evaporation is effectively cut off. Moisture continues to be drawn upward until a balanced condition is developed. During this process the added moisture causes the soil to swell, forcing the slab upward. Since evaporation takes place around the perimeter of the slab, the soil near the edge of the slab does not swell as much as the soil under the center portion of the slab. This condition causes differential movement and cracking of the slab.

Several preventive measures are available to reduce the effects of volume changes in

these soils. One is to use deeper grade beams to provide a barrier to evaporation of water from below the slab. Another is to place a paved strip around the perimeter of the building. This strip acts as a buffer zone, with most of the differential movement taking place in the area. A minimum width of 5 feet is normally recommended. Residences or other structures may use a mulch bed around the perimeter to help keep moisture from evaporating. Lime stabilization at a 5 foot wide strip outside the building line will also help prevent moisture loss.

Other steps to prevent the effects of moisture changes are to extend roof drains down spouts to 5' from the slab; to provide a good sprinkler system around the slab to avoid excessive drying, and to further increase the non expansive fill under the slab.

Trees can also contribute to the soil shrink/swell movement in highly plastic soils. During extended periods of dry weather, trees remove water from the soil and cause shrinkage. This shrinkage causes movement of the soil downward and toward the tree and can seriously damage nearby structures. This condition can normally be neutralized by removing the trees or by placing the structure on foundation bearing below the affected soil.

Drainage

Site grading should promote positive drainage away from the building site during and after construction (sand should not be used as fill for site grading). Ponding should not be allowed near the foundation soils. Also, the building subgrade should be sealed against water intrusion along trenches and conduits created for utility access under the building.

The upper part of utility excavation should be backfilled with properly compacted clayey soil to reduce and control infiltration of water. A clay plug should be provided in the trench on the exterior of the building to prevent water from gaining access along the trench to the subgrade beneath the structure.

Where paving and flatwork abuts the structure, care should be taken that the joint is properly sealed and maintained. Roof drains should discharge on pavement or away from the foundations.

Vegetation

Tree roots remove moisture from the soil. When trees are removed from a construction site, a zone of desiccated soil is left underneath which would gain moisture for many years. If the soil is expansive this will result in expansion. The process has been known to last for as long as 20 years. In such a situation a foundation for the structure must be designed for the anticipated heave. Alternately, the construction on the site should be postponed for several years to allow moisture stabilization.

New trees should not be planted closer than half the canopy diameter of a mature tree. Alternately, root barriers must be placed near the exterior grade beams.

If sprinkler systems are used they must be installed all around the structure for uniformity.

Additional Fill Soils and Placement

All fill soil should be a sandy material with a Liquid Limit below 40 and a Plasticity Index between 7 and 20. To provide proper support for the foundation, it is essential that the fill be properly compacted. The fill should be placed in maximum lifts of eight inches, measured loose, and compacted to a minimum of 95 percent of the optimum density as obtained by the Standard Proctor Test, ASTM D-698.

The slab should be bedded on a layer of sand approximately two inches in thickness. A layer of six millimeter plastic sheeting should be used below the sand to prevent moisture migration.

GENERAL

Reasonable variations from the subsurface information presented in this report are assumed. If conditions encountered during construction are significantly different than those represented in this report, Coastal Testing Laboratories, Inc. should be notified immediately.

A Ground Fault Study is not part of this investigation.

The following Plates are attached at the end of the text and complete this report.

Plate 1	Boring Location Plan
Plates 2-3	Boring Logs/Tests Results
Plate 4	Key to Soil Symbols

SITE INVESTIGATION

Exploration borings were drilled to evaluate the subsurface soil conditions at the locations shown on Plate 1. These borings were performed with a truck mounted, power driven auger-drilling rig. Cohesive soils were sampled by pushing thin-wall steel core barrels (Shelby Tubes) into natural soil at the bottom of each boring as the drilling progressed (ASTM D-1587). The penetration resistance of each soil sample was evaluated in the field with a field Penetrometer.

Where encountered, cohesionless soils are sampled by means of a split spoon sampler using the Standard Penetration Test procedure (ASTM D-1596). In this method of sampling, the sampler is driven into the soil by means of a 140 lb. hammer dropping thirty inches. The number of such blows required to drive the sampler twelve inches after being seated six inches in the soil is recorded as the penetration resistance in blows per foot. This number is modified in the office to include the effect of overburden as proposed by Bazaraa¹. The modified number is presented in the Logs as N, the penetration resistance in blows per foot. Samples are taken of the contents of the split spoon sampler in each case for further examination in the laboratory.

Samples were taken at intermittent depths from all formations pertinent to the design of the floor system and structural foundations.

¹Bazaraa A. R. (1967) "Use of Standard Penetration Test for Estimating Settlements of Shallow Foundations on Sand" Phd. Thesis, Univ. of Illinois, Urbana, 379 pp.

LABORATORY TESTING

Subsequent to the completion of the borings, the samples were returned to our laboratory for visual and textural examination, soil classification (ASTM D-2487) and testing.

All samples were tested to determine their natural moisture content (ASTM D-2216). Additional tests on selected samples include Sieve Analysis (ASTM D-422), Atterberg Limits (ASTM D-4318) and Unconfined Compression and Density (ASTM D-2166).

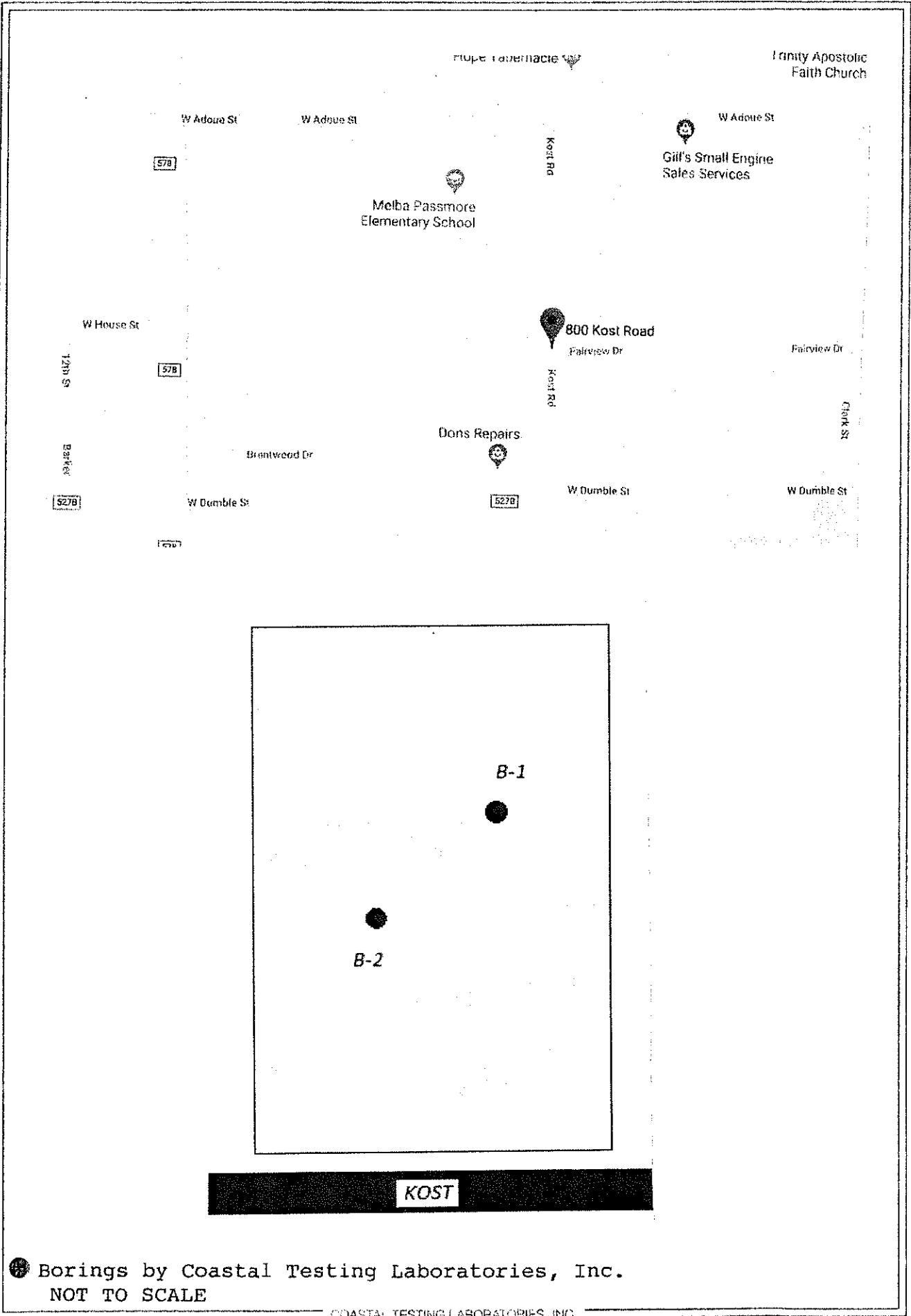
The results of all tests are summarized at the end of the text. The soil samples will be retained in our storage facility for thirty days unless otherwise advised.

The results of the laboratory tests were incorporated into an engineering analysis. The evaluation of these results and observations made in the field form the basis for recommendations contained herein.



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186



● Borings by Coastal Testing Laboratories, Inc.
NOT TO SCALE

LOG OF BORING NO. 1

Job No:18-422 Report No:1812/1322 Date: 12/04/18 Driller:P. Stephens
 Complete at: 20' Boring Method: 4" Auger, 3" Shelby Tube
 Water Encountered:13' Water at Completion:16'
 Location: 800 Kost Street. Alvin, TX 77511

Ft.	ST	MC	LL	PI	Dd	GS	TV	HP	Qu	Stratum Description
— A		31.6	79	54				1.0		Dark Gray Clay
— A		33.3						1.0		
5' — A		35.7			87.5			2.5	1.07	
— A		36.8	93	68				1.5		Red @6'
— A		35.5						2.0		
10'										
										Red Very Sandy Clay (CL-ML)
— A		23.1						2.0		
15'										
										Red Silty Sand (SM)
— A		25.5								
20'										

A = Shelby Tube	ST = Standard Penetration Test	R = red	sa = sandy	lt = light tan
B = Split Spoon	HP = Hand Penetrometer tsf	G = gray	si = silty	fn = ferrous nodules
D = Rock Core	TV = Torvane Shear Strength tsf	T = tan	cl = clay	cn = calcareous nodules
E = No Recovery	GS = % Finer #200 Sieve	w. = with	lo = iron ore	cs = calcium sulfat
MC = Moisture Content	Qu = Unconfined Compression tsf	v. = very	org = organics	lg = light gray
LL = Liquid Limit	PI = Plasticity Index	dg = dark gray	cl = clayey	
Dd = Dry Density pcf	SLS = Slickensided			

LOG OF BORING NO. 2

Job No:18-422 Report No:1812/1322 Date: 12/04/18 Driller:P. Stephens
 Complete at: 20' Boring Method: 4" Auger, 3" Shelby Tube
 Water Encountered:13' Water at Completion:16'
 Location: 800 Kost Street. Alvin, TX 77511

Ft.	ST	MC	LL	PI	Dd	GS	TV	HP	Q _u	Stratum Description
— A		29.3						1.5	3.18	Dark Gray Clay (CH)
— A		24.5	92	65				4.5		
5' — A		26.5	85.3					4.5		Gray @ 5'
— A		33.6						1.5		
— A		44.0	90	63				1.5		
10'										
— A		24.6						4.0		Very Sandy Clay (CL-ML)
15'										
— A		28.3								Red Silty Sand (SM)
20'										

A = Shelby Tube	ST = Standard Penetration Test	R = red	sa = sandy	lt = light tan
B = Split Spoon	HP = Hand Penetrometer tsf	G = gray	si = silty	fn = ferrous nodules
D = Rock Core	TV = Torvane Shear Strength tsf	T = tan	cl = clay	cn = calcareous nodules
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LL = Liquid Limit	PI = Plasticity Index	dg = dark gray	cl = clayey	
Dd = Dry Density pcf	SLS = Slickensided			

**KEY TO SOIL DESCRIPTIONS
USED IN LABORATORY LOGS**

COLOR

In color description of sample, the predominate color is stated first.

CONSISTENCY OF COHESIVE SOILS

Field Identification	Laboratory Identification
Very soft - Tall core will slump	0 - 0.25 tsf
Soft - Core can be pinched	0.25 - 0.50 tsf
Firm - Easily imprinted with fingers	0.50 - 1.00 tsf
Stiff - Can be imprinted with fingers	1.00 - 2.00 tsf
Very stiff - Can be imprinted very slightly with fingers	2.00 - 4.00 tsf
Hard - Can not be imprinted with fingers	Over 4.00 tsf

RELATIVE DENSITY OF COHESIONLESS SOILS

Classification by Standard Penetration Resistance:

Loose - 0 - 10 blows/ft	Dense - 30 - 50 blows/ft
Med.Dense - 10 - 30 blows/ft	Very Dense - 50 and above blows/ft

SOIL STRUCTURE

SLS = Slickensided - Cut by old fracture planes which are slick and glossy.

FRC = Fractured - Containing cracks, filled with various materials.

VVD = Varved - Composed of thin laminae of varying color and soil types.

INB = Interbedded - Composed of alternate layers of different soil types.

CLC OR CN = Calcareous - Contains deposits of calcium carbonate.