LSTL

LONE STAR TESTING LABORATORIES

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June 18, 2015

Primehome Design & Construction
911 West 21st Street
Houston, Texas 77008

Attn: Philip Tijerina

Re: Soil Foundation Investigation
Residence at 27035 Star Gazer Way
Lot 9, Block 1, Section 7 - Benders Landing Estates S/D
Montgomery County, Texas

Project No.: 1506-009
Report No.: 1506009-1

Dear Philip,

We are pleased to submit this report on the soil foundation investigation made recently at the site referred to above.

This investigation reveals non-plastic fine sand for the entire formation explored. This soil is suitable for slab-on-ground/fill floor slabs with considerations as addressed in the report.

For a pier & beam design, spread footings are recommended, founded in the clayey sand at or below 2 to 4 feet of depth, and proportioned for a safe bearing capacity of 1500 PSF for total dead and live loads. Parameters for a shallow foundation system such as a post-tensioned slab or a waffle type slab are addressed in the report for the use of your designer.

It has been a pleasure being of service to you on this project. If we may be of any further assistance, please call us.

Respectfully,

James Hickey

James L. Hickey, P. E.

6.18.15
JAMES L. HICKEY
30518

5815 BENDER ROAD • HUMBLE, TEXAS 77396

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PLATES AND CHARTS

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SUBJECT: REPORT OF SOIL FOUNDATION INVESTIGATION

RESIDENCE AT 27035 STAR GAZER WAY
LOT 9, BLOCK 1, SECTION 7
BENDERS LANDING ESTATES S/D
SOUTHEAST MONTGOMERY COUNTY, TEXAS

TO: PRIMEHOME DESIGN & CONSTRUCTION
911 WEST 21ST STREET
HOUSTON, TEXAS 77008

ATTN: PHILIP TIJERINA

SCOPE AND PURPOSE:

This report presents the results of the foundation investigation made recently at the subject site to determine the nature and condition of surface and sub-surface soil as affects the design of foundations. In particular, it was desirable to determine the feasibility of slab-on-ground/fill type first floor construction, depth to water table where encountered, optimum type and depth of structural foundations and safe soil bearing capacity. The investigation was made in accordance with your instructions.

PROCEDURES: FIELD

Two (2) borings were made to a depth of 15 feet each at the locations shown on the Location of Test Borings plate or Figure 1. The borings were made with a Lone Star 320 rotary mobile drill rig using no drilling water in order to secure unaffected soil samples and reliable data on groundwater levels. The soil was sampled by pushing a thin-walled Shelby tube sampler into the soil in accordance with ASTM Specification D 1587-74. The relative density of the sand was determined by noting the resistance to penetration of the samplers as in the Standard Penetration Test. The samples were taken by a geotechnical engineering aide who noted the consistency, color, composition, and classification of the soil as encountered.

The unconfined compressive strength of the cohesive soil was measured in the field by use of a Soiltest Cl-700 Penetrometer. This value is reported on the logs of borings.

The samples were examined and classified in accordance with the Unified Soil Classification System. They were then sealed to prevent moisture loss and transported to the laboratory for subsequent testing.

PROCEDURES: LABORATORY

In the laboratory, the samples were tested for moisture contents, density, unconfined compressive strength, and Atterberg limits. The final logs of borings were prepared by a geotechnical engineer after examining the samples, and reviewing the results of tests. The results of these tests are shown on the Logs of Borings.

PROJECT DESCRIPTION AND AUTHORIZATION:

The project consists of a 4,100 square foot, 2-story residence, with attached garage, on a concrete slab, with wood frame. Wall loads and pier loads are not known at this time, but are not expected to exceed 2 kips per foot, and 40 Kips, respectively. The soil investigation was requested by Philip Tijerina with Primehome Design & Construction, the builder.

GEOLOGY:

The surficial soil at this site is underlain by the Willis formation of the Pleistocene era. This formation consists of overconsolidated clay, silts, and sand with shell fragments, calcium carbonates, and ferrous oxides. These formations tend to extend to a depth of about 100 feet, and are quite strong; although the surface has been weakened somewhat by the weathering process.

A fault study is beyond the scope of this investigation. For information on area faulting, it is recommended that a professional geologist be consulted.

SITE DESCRIPTION:

The sites consists of a 4-sided (1 side is partially curved) wooded corner lot fronting at 27035 Star Gazer Way (at Hidden Grove Landing Drive) in the Benders Landing Estates Subdivision in Southeast Montgomery County, Texas. The lot slopes toward the roads, and the surface sand was drained at the time of the investigation.

VARIATIONS:

The recommendations contained in this report are based on data gained from the test borings at the location shown on the Location of Test Boring plate, Figure 1, a reasonable volume of laboratory tests, and professional interpretation and evaluation of this data in view of the project information provided this firm. Should soil conditions differing from those described in this report be encountered at other locations in the course of construction, or should the design data change significantly, this firm should be notified immediately so that the conditions and their effect may be evaluated.

SOIL STRATIGRAPHY:

The soil consists of medium dense brown non-plastic sand (SM), wet & saturated below 8 feet, extending to the maximum depth of the borings at 15 feet. A more detailed stratigraphy can be seen on the logs of borings.

Water was encountered at 11 & 12 feet of depth during the boring operations. However, it should be noted that ground water levels are subject to the influence of seasonal variations as well as other factors and should be checked prior to the initiation of any construction that could be affected.

ENGINEERING ANALYSIS:

The expansive potential of the surface and shallow formations was determined by comparison of the natural moisture content of the soil with the results of Atterberg limit tests. Experience has shown that plastic soil having moisture contents equal to or less than the plastic limit of the soil is potentially expansive with the expansion pressure varying directly with the plasticity index and inversely with the moisture content. On the other hand, soil having low or moderate plasticity indices and moisture content above the plastic limit is essentially non-expansive. Soil with high plasticity indices is practically always subject to volume changes regardless of the moisture.

Safe soil bearing pressures for cohesive formations are calculated from the depth and undrained shear strength of the soil determined by unconfined compression tests and field penetrometer values. Safe soil bearing pressures for cohesionless soil are determined from the values established by the Standard Penetration Test and interpretation of these values. A safety factor of two (2) is used for total dead and live load. A safety factor of three (3) is used for dead load and sustained live load. The most suitable type of foundation is determined by review of the job requirements, the logs of borings, and the test results. The most suitable depth is selected as the minimum depth below the zone of seasonal moisture fluctuations affording reasonably uniform footing support, reasonably high safe bearing capacities, and adequate vertical clearance with physical features of the proposed structures.

Surficial soil is studied for the ease of compactability and manipulation in the field during construction. Also, should the site have poor soil or should drainage conditions be restricted, consideration is given to the alternatives for stabilization or removal and replacement of the surficial soil with select compactible soil. These are some of the considerations given to pavement design.

Certain tests are performed for building conditions in which certain characteristics of the soil are critical to the design of the structure. When long-term settlement analysis is required, consolidation tests are performed. Triaxial tests are performed to measure shear strength and pore pressure in sandier soil. Permeability tests are performed when the loss of fluids through the soil is critical. However, these are not critical tests for this project.

SITE PREPARATION:

NOTE: Positive drainage and prevention of erosion of the surface soil are essential for this site. The soil consists of non-plastic sand (SM) that can become unstable during wet weather, and should be shaped to drain, be stabilized (as deep as necessary), and be compacted, or be removed and replaced with compacted select fill to a minimum depth of 2 feet, after positive proof-rolling of the underlying soil. Interceptor ditches or swales should be constructed to intercept surface water and direct the same away from the building site.

It is recommended that the following procedures be implemented in preparation of the site for construction:

1) Strip and scarify the surface soil to a minimum depth of six (6) inches and remove all surface organics, trash, debris, and other deleterious materials. If trees are to be removed, the root system should be removed to a minimum depth of 2 feet or to a depth where the maximum root size is less than 1/2 inch.

2) Provide positive drainage by sloping, and directing the runoff away from the building. This includes all roof drain downspouts after construction extending the outfall of the same beyond the residence pad.

3) Proof-roll the prepared soil with a loaded dump truck to locate any wet or pumping area and treat the same with the proper stabilizing agents. Compact the soil to 100 percent of natural density (No ruts when proof-rolled with a loaded dump truck or equivalent).

4) Any fill required under floor slabs in the building area should be a select soil consisting of sandy and/or silty clay free of any organics, trash, or other deleterious materials with a minimum liquid limit of 25. The plasticity index (PI) should range from ten (10) to twenty (20). Compact the select fill in six (6) inch lifts to ninety-five (95) percent of Standard Proctor Density, in conformance with the standard procedure, ASTM D 698, at or within three (3) percent of optimum moisture.

5) The building pad should consist of a minimum of 12 inches of compacted select fill, or more if necessary for proper drainage. The pad should extend a minimum of 3 feet beyond the periphery of the residence, if space allows. The placement of the fill should be monitored by this firm or another approved geotechnical engineering firm. The maximum slope of select fill should not exceed a 3:1 (horizontal to vertical) slope, and 5:1 for sand.

FOUNDATION CONSIDERATIONS:

For a pier & beam design, individual spread footings are recommended. The recommended slab for shallow foundations is an engineered post-tensioned slab or an FHA Type III waffle slab as outlined in the Wire Reinforcement Institute, Inc. publication DESIGN OF SLAB ON GROUND FOUNDATIONS. Effective PI = 15 (For FHA & B.R.A.B. Report #33 design only). The climatic rating, Cw = 25.

The Post-Tensioning Institute, Inc. parameters for the DESIGN AND CONSTRUCTION OF POST-TENSIONED SLABS-ON-GROUND, 3rd Edition classifies this site as a Non Active Site, Item 3.2.3, and recommends that the B.R.A.B. Report #33 parameters be used.

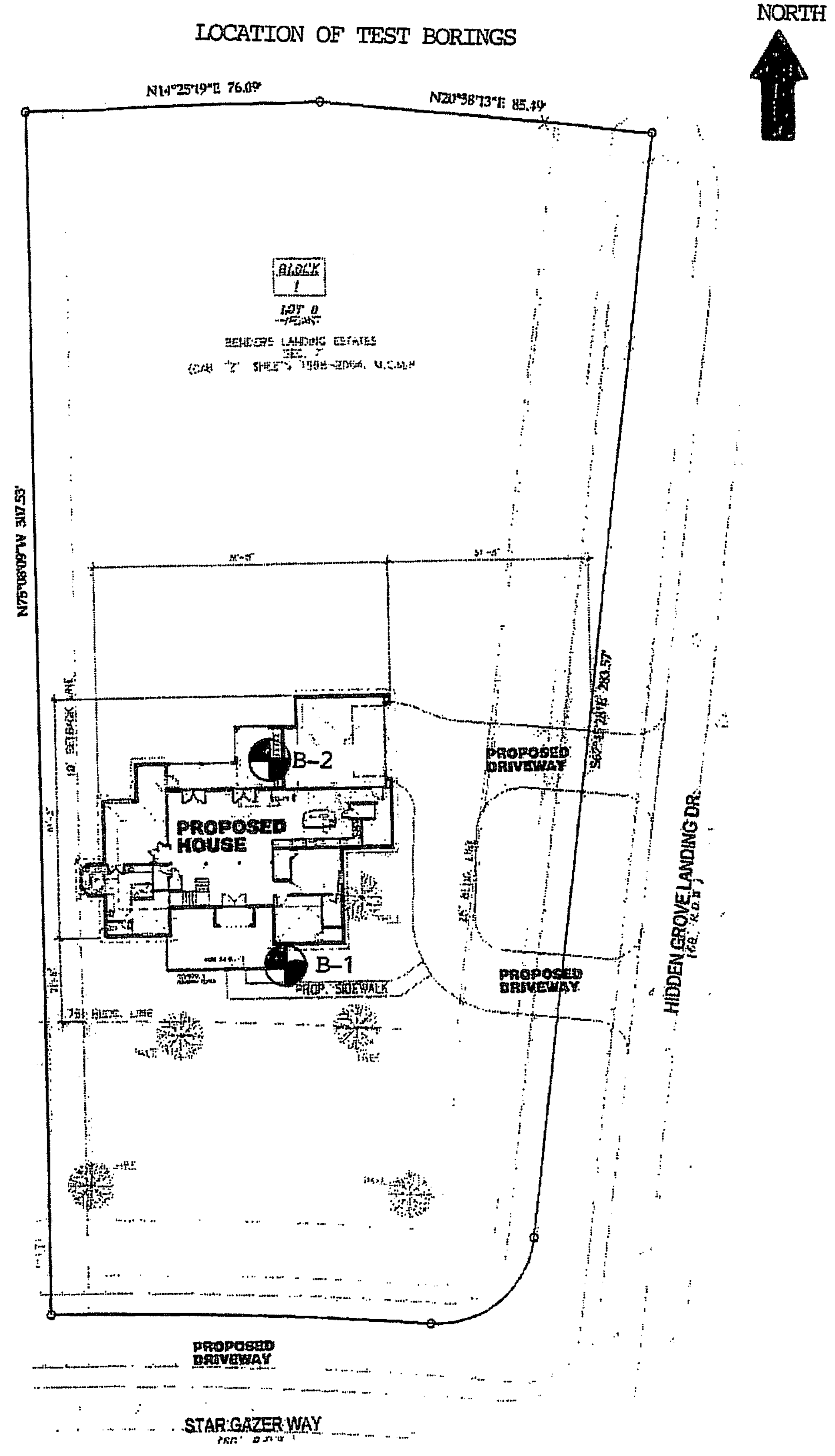
SHALLOW FOUNDATIONS:

After proper stabilization of the sand, as required, continuous footings (beams) founded in the sand should be proportioned for a safe bearing capacity of 800 PSF, incorporating a minimum safety factor of 3. This value can increase to 1200 PSF, if the grade beams are founded on a minimum of 12 inches of compacted select fill verified & tested by Lone Star Testing Laboratories.

SPREAD FOOTINGS:

Individual spread (dug) footings should be founded in the sand at the 2 to 4 foot depth, below existing grade, and be proportioned for a safe bearing capacity of 1500 PSF for total dead and live loads incorporating a minimum safety factor of 2. For total dead and sustained live loads, the safe bearing capacity is 1000 PSF incorporating a minimum safety factor of 3.

LOCATION OF TEST BORINGS



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Not to Scale

Figure 1

LOG OF BORING
BORING NO: B-1

PROJECT: Residence @ 27035 Star Gazer Way
FOR: Primehome Design & Construction
DATE: 6-10-2015
DRILLER: Knight Drilling

JOB NO: 1506-009
BORING METHOD: Core
AUGER: X
WASH:
GROUND ELEV: Existing

Depth (Feet)	Sample Method	Water Levels	Penetrometer or Blow Count	Compressive Strength Tons/Sq. Ft.	Moisture Content (%)	Dry Density Lbs./Cu. Ft.	Liquid Limit %	Plasticity Index	■ Shelby Tube ☒ Standard Penetration Test ▨ No Recovery ▽ Initial Water Level ▼ Water Level After
	■		10		5				Medium, brown sand (SM)
	■		12		6			NP	...tan
5	■		13		7				...same
	■		17		10			NP	...same
	■		19		15				...saturated
10		▽							
	■		16		19				...same
15									

Boring terminated at 15'
Water encountered at 11'

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LOG OF BORING
BORING NO: B-2

PROJECT: Residence @ 27035 Star Gazer Way
FOR: Primehome Design & Construction

DATE: 6-10-2015
DRILLER: Knight Drilling

JOB NO: 1506-009
BORING METHOD: Core
AUGER: X
WASH:
GROUND ELEV: Existing

■ Shelby Tube
☒ Standard Penetration Test
▨ No Recovery
▽ Initial Water Level
▼ Water Level After

Depth (Feet)	Sample Method	Water Levels	Penetrometer or Blow Count	Compressive Strength Tons/Sq. Ft.	Moisture Content (%)	Dry Density Lbs./Cu. Ft.	Liquid Limit %	Plasticity Index	
	■		11		5			NP	Medium, brown sand (SM)
	■		11		5				...tan
5	■		14		7			NP	...same
	■		19		11				...same
	■		21		12				...same
10									
		▽							
	■		17		17				...saturated
15									

Boring terminated at 15'
Water encountered at 12'

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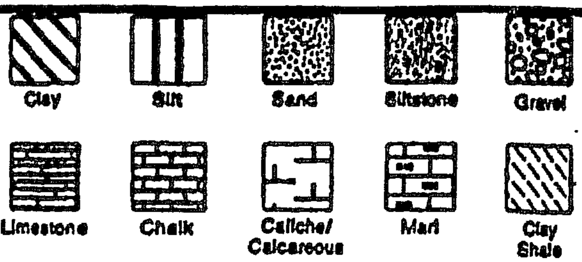
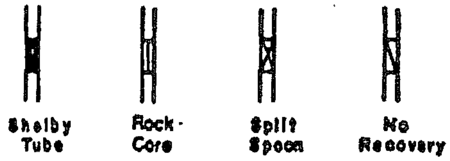
SYMBOLS AND TERMS USED ON BORING LOGS



SAMPLER TYPES

(SHOWN IN SAMPLES COLUMN)



TERMS DESCRIBING CONSISTENCY OR CONDITION

COARSE GRAINED SOILS (Major Portion Retained on No.200 Sieve): includes (1) clean gravels and sands, and (2) silty or clayey gravels and sands. Condition is rated according to relative density, as determined by laboratory tests.

Descriptive Term	Standard Penetration, Resistance, Blows/Ft	Relative Density
Loose	0 - 10	0 to 40%
Medium dense	10 - 30	40 to 70%
Dense	30 - 50	70 to 100%

FINE GRAINED SOILS (Major portion passing No. 200 sieve): includes (1) inorganic and organic silts and clays, (2) gravelly, sandy, or silty clays, and (3) clayey silts. Consistency is rated according to shearing strength, as indicated by penetrometer readings or by unconfined compression tests.

DESCRIPTIVE TERM	UNCONFINED COMPRESSIVE STRENGTH TONS/Sq.Ft.
Very soft	less than 0.25
Soft	0.25 to 0.50
Firm	0.50 to 1.00
Stiff	1.00 to 2.00
Very Stiff	2.00 to 4.00
Hard	4.00 and higher

Note: Slickensided and fissured clays may have lower unconfined compressive strengths than shown above, because of planes of weakness or cracks in the soil. The consistency ratings of such soils are based on penetrometer readings.

TERMS CHARACTERIZING SOIL STRUCTURE

Parting: -paper thin in size Seam: -1/8"-3" thick Layer: -greater than 3"

Slickensided	- having inclined planes of weakness that are slick and glossy in appearance.
Fissured	- containing shrinkage cracks, frequently filled with fine sand or silt; usually more or less vertical.
Laminated	- composed of thin layers of varying color and texture.
Interbedded	- composed of alternate layers of different soil types.
Calcareous	- containing appreciable quantities of calcium carbonate.
Well graded	- having wide range in grain sizes and substantial amounts of all intermediate particle sizes.
Poorly graded	- predominantly of one grain size, or having a range of sizes with some intermediate size missing.
Flocculated	- pertaining to cohesive soils that exhibit a loose knit or flakey structure.

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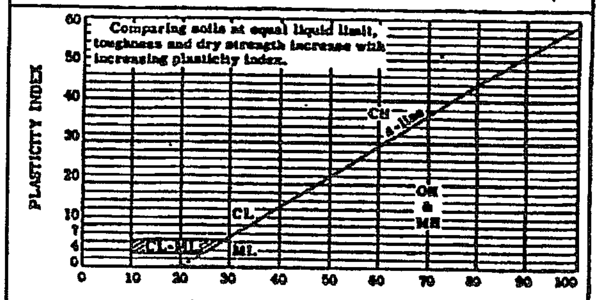
UNIFIED SOIL CLASSIFICATION SYSTEM

Major Divisions			Group Symbols	Typical Names	Field Identification Procedures (excluding particles larger than 3 in. and basing fractions on estimated weights)			Information Required for Describing Soils	Laboratory Classification Criteria			
(1)	(2)		(3)	(4)	(5)			(6)	(7)			
Coarse-grained Soils (More than half of material is larger than No. 200-sieve size.) (The No. 200-sieve size is about the smallest particle visible to the naked eye.)	Gravels (More than half of coarse fraction is larger than No. 4-sieve size.) (For visual classification, the 1/4-in. size may be used as equivalent to the No. 4-sieve size.)	Clean Gravels (Little or no fines)	GW	Well-graded gravels, gravel-sand mixtures, little or no fines	Wide range in grain sizes and substantial amounts of all intermediate particle sizes			For undisturbed soils, add information on stratification, degree of compactness, cementation, moisture conditions, and drainage characteristics. Give typical name; indicate approximate percentages of sand and gravel; maximum size; angularity, surface condition and hardness of the coarse grains; local or geologic name and other pertinent descriptive information; and symbol in parentheses. Example: Silty sand, gravelly; about 20% hard, angular gravel; particles 1/2-in. maximum size; rounded and subangular sand grains coarse to fine; about 15% nonplastic fines with low dry strength; well compacted and moist in place; alluvial sand; (SM).	Use grain-size curve in identifying the fractions as given under field identification.	Determine percentages of gravel and sand from grain-size curve. Depending on percentage of fines (fraction smaller than No. 200-sieve size), coarse-grained soils are classified as follows: Less than 5% GW, GP, SW, SP More than 12% GM, GC, SM, SC 5% to 12% Borderline cases requiring use of dual symbols.	$C_u = \frac{D_{60}}{D_{10}}$ (greater than 4) $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ (between one and 3)	
			GP	Poorly graded gravels, gravel-sand mixtures, little or no fines	Predominantly one size or a range of sizes with some intermediate sizes missing						Not meeting all gradation requirements for GW	
		Gravels with Fines (Appreciable amount of fines)	GM	Silty gravels, gravel-sand-silt mixtures	Nonplastic fines or fines with low plasticity (for identification procedures see ML below)						Atterberg limits below A-line or PI less than 4	Above A-line with PI between 4 and 7 are borderline cases requiring use of dual symbols.
			GC	Clayey gravels, gravel-sand-clay mixtures	Plastic fines (for identification procedures see CL below)						Atterberg limits above A-line with PI greater than 7	
	Sands (More than half of coarse fraction is smaller than No. 4-sieve size.)	Clean Sands (Little or no fines)	SW	Well-graded sands, gravelly sands, little or no fines	Wide range in grain size and substantial amounts of all intermediate particle sizes						$C_u = \frac{D_{60}}{D_{10}}$ (greater than 6) $C_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ (between one and 3)	
			SP	Poorly graded sands, gravelly sands, little or no fines	Predominantly one size or a range of sizes with some intermediate sizes missing						Not meeting all gradation requirements for SW	
		Sands with Fines (Appreciable amount of fines)	SM	Silty sands, sand-silt mixtures	Nonplastic fines or fines with low plasticity (for identification procedures see ML below)						Atterberg limits below A-line or PI less than 4	Limits plotting in hatched zone with PI between 4 and 7 are borderline cases requiring use of dual symbols.
			SC	Clayey sands, sand-clay mixtures	Plastic fines (for identification procedures see CL below)						Atterberg limits above A-line with PI greater than 7	
					Identification Procedures on Fraction Smaller than No. 40-Sieve Size							
					Dry Strength (Crushing characteristics)	Dilatancy (Reaction to shaking)	Toughness (Consistency near PL)					
Fine-grained Soils (More than half of material is smaller than No. 200-sieve size.)	Silts and Clays (Liquid limit less than 50)		ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands, or clayey silts with slight plasticity	None to slight	Quick to slow	None	Give typical name; indicate degree and character of plasticity; amount and maximum size of coarse grains; color in wet condition, odor if any, local or geologic name, and other pertinent descriptive information; and symbol in parentheses. For undisturbed soils add information on structure, stratification, consistency in undisturbed and remolded states, moisture and drainage conditions. Example: Clayey silt, brown, slightly plastic, small percentage of fine sand, numerous vertical root holes, firm and dry in place, loess, (ML).		 Comparing soils at equal liquid limit, toughness and dry strength increase with increasing plasticity index. Liquid Limit Plasticity Chart (for laboratory classification of fine-grained soils)		
			CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays	Medium to high	None to very slow	Medium					
			OL	Organic silts and organic silty clays of low plasticity	Slight to medium	Slow	Slight					
	Silts and Clays (Liquid limit greater than 50)		MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts	Slight to medium	Slow to none	Slight to medium					
			CH	Inorganic clays of high plasticity, fat clays	High to very high	None	High					
			OH	Organic clays of medium to high plasticity, organic silts	Medium to high	None to very slow	Slight to medium					
Highly Organic Soils			Pt	Peat and other highly organic soils	Readily identified by color, odor, spongy feel, and frequently by fibrous texture							

(1) Boundary classifications: Soils possessing characteristics of two groups are designated by combinations of group symbols, for example, GW-GC, well-graded gravel-sand mixture with clay binder.
(2) All sieve sizes on this chart are U. S. standard.

Field Identification Procedures for Fine-grained Soils or Fractions

These procedures are to be performed on the minus No. 40-sieve-size particles, approximately 1/64 in. For field classification purposes, screening is not intended; simply remove by hand the coarse particles that interfere with the tests.

Dilatancy (reaction to shaking)

After removing particles larger than No. 40-sieve size, prepare a pat of moist soil with a volume of about 1/2 cu. in. Add enough water if necessary to make the soil soft but not sticky.

Place the pat in the open palm of one hand and shake horizontally, striking vigorously against the other hand several times. A positive reaction consists of the appearance of water on the surface of the pat, which changes to a livery consistency and becomes glossy. When the sample is squeezed between the fingers, the water and gloss disappear from the surface, the pat stiffens, and finally it cracks or crumbles. The rapidity of appearance of water during shaking and of its disappearance during squeezing assist in identifying the character of the fines in a soil.

Very fine clean sands give the quickest and most distinct reaction, whereas a plastic clay has no reaction. Inorganic silts, such as a typical rock flour, show a moderately quick reaction.

Dry Strength (crushing characteristics)

After removing particles larger than No. 40-sieve size, mold a pat of soil to the consistency of putty, adding water if necessary. Allow the pat to dry completely by oven, sun, or air drying, and then test its strength by breaking and crumbling it between the fingers. This strength is a measure of the character and quantity of the colloidal fraction contained in the soil. The dry strength increases with increasing plasticity.

High dry strength is characteristic for clays of the CH group. A typical inorganic silt possesses only very slight dry strength. Silty fine sands and silts have about the same slight dry strength but can be distinguished by the feel when powdering the dried specimen. Fine sand feels gritty, whereas a typical silt has the smooth feel of flour.

Toughness (consistency near plastic limit)

After removing particles larger than the No. 40-sieve size, a specimen of soil about 1/2-in. cube in size is molded to the consistency of putty. If too dry, water must be added, and if sticky, the specimen should be spread out in a thin layer and allowed to lose some moisture by evaporation. Then the specimen is rolled out by hand on a smooth surface or between the palms into a thread about 1/8 in. in diameter. The thread is then folded and rerolled repeatedly. During this manipulation the moisture content is gradually reduced and the specimen stiffens, finally loses its plasticity, and crumbles when the plastic limit is reached.

After the thread crumbles, the pieces should be lumped together and a slight kneading action continued until the lump crumbles.

The tougher the thread near the plastic limit and the stiffer the lump when it finally crumbles, the more potent is the colloidal clay fraction in the soil. Weakness of the thread at the plastic limit and quick loss of coherence of the lump below the plastic limit indicate either inorganic clay of low plasticity or materials such as kaolin-type clays and organic clays which occur below the A-line.

Highly organic clays have a very weak and spongy feel at the plastic limit.

Adopted by Corps of Engineers and Bureau of Reclamation, January, 1952.