

#### **GENERAL NOTES**

12 MATERIALS

1.1 FABRICATION SHALL BE IN ACCORDANCE WITH METAL BUILDING SUPPLIER, STANDARD PRACTICES IN COMPLIANCE WITH THE APPLICABLE SECTIONS, RELATING TO DESIGN REQUIREMENTS AND ALLOWABLE STRESSES OF THE LATEST EDITION OF THE "AWS STRUCTURAL WELDING CODE D1.1"

1.2 FABRICATION SHALL BE IN ACCORDANCE WITH METAL BUILDING SUPPLIER. STANDARD PRACTICES IN CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO CONCERN AND CON

ASTM DESIGNATION MIN YIELD STRENGTH

1.2	MATERIALS	TIOTIM DEGICITATION	MIN. HELD SHIEROTH
l	HOT ROLLED STEEL SHAPES (W, & C)	A572	Fy = 50 KSI
l	HOT ROLLED STEEL ANGLES (L)	A36	Fy = 36  KSI
l	STEEL PIPES	A500	Fy = 42 KSI
l	STRUCTURAL TUBING	A500	Fy = 42  KSI
l	STRUCTURAL STEEL WEB PLATE	A572/A1011	Fy = 50 KSI
l	STRUCTURAL STEEL FLANGE PLATES/BARS	A529/A572	Fy = 55  KSI
l	COLD FORMED LIGHT GAGE	A653/A1011	Fy = 55  KSI
l	ROOF & WALL SHEETS	A792/A653	$F_{V} = 50, 80 \text{ KSI}$
l	CABLE BRACE	A475 - TYPE 1	EXTRA HÌGH STRENGTH
	ROD BRACE	A36	Fy = 36 KSI
			MIN. TENSILE STRENGTH
l	MACHINE BOLTS & NUTS	A307	Fu = 60 KSI
l	HIGH STRENGTH BOLTS (1" & LESS)	A325-TYPE 1	Fu = 120 KSI

PRIMER
SHOP PRIMER PAINT IS A RUST INHIBITIVE PRIMER WHICH MEETS THE END PERFORMANCE OF
FEDERAL SPECIFICATION SSPC NO. 15 AND IS GRAY OXIDE IN COLOR. THIS PAINT IS NOT
INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS. METAL BUILDING SUPPLIER IS NOT
RESPONSIBLE FOR ANY DETERIORATION OF THE SHOP PRIMER PAINT AS A RESULT OF
IMPROPER HANDLING AND/OR JOBSITE STORAGE. METAL BUILDING SUPPLIER SHALL NOT BE
RESPONSIBLE FOR ANY FIELD APPLIED PAINT AND/OR COATINGS.
(AISC CODE OF STANDARD PRACTICE, LATEST EDITION).
NOMINAL THICKNESS OF PRIMER WILL BE 1 MIL UNLESS OTHERWISE SPECIFIED IN CONTRACT
DOCUMENTS.

1.4 GALVANIZED OR SPECIAL COATINGS: SEE CONTRACT DOCUMENTS

1.5 ALL BOLTS ARE 1/2"ø x 0'-1 1/4" A307 EXCEPT :

HIGH STRENGTH BOLTS (>1"ø TO 1 1/2"ø) A325—TYPE 1 ANCHOR BOLTS (NOT SUPPLIED BY M.B.S.) A36/A307/F1554

A) ENDWALL RAFTER SPLICE - 5/8" Ø x 0"-1 3/4" A325-N B) ENDWALL COLUMN TO RAFTER CONNECTION - (SEE WALL ELEVATION) C) MAIN FRAME CONNECTIONS - SEE CROSS SECTION D) FLANGE BRACECONNECTIONS - 1/2" Ø x 0"-1 1/4" A325

NOTE: WASHERS ARE NOT SUPPLIED UNLESS NOTED OTHERWISE ON DRAWING

#### 1.6 A325 BOLT TIGHTENING REQUIREMENTS

ALL HIGH STRENGTH BOLTS ARE A325—N UNLESS SPECIFICALLY NOTED OTHERWISE. HOLES ARE NOT SLOTTED AND DESIGN IS BEARING CONNECTION.
STRUCTURAL BOLTS SHALL BE TIGHTENED BY THE "TURN-OF-THE-NUT" METHOD IN ACCORDANCE WITH THE LATEST EDITION AISC "SPECIFICATION FOR STRUCTURAL JOINTS" USING ASTM A325 OR A490 BOLTS, WHEN SPECIFICALLY REQUIRED. A325—N BOLTS ARE SUPPLIED WITHOUT WASHER UNLESS OTHERWISE NOTED ON THE DRAWINGS.

ALL BOLTED CONNECTIONS UNLESS NOTED ARE DESIGNED AS BEARING TYPE CONNECTIONS WITH BOLT THREADS NOT EXCLUDED FROM THE SHEAR PLANE

- 1.7 CLOSURE STRIPS ARE FURNISHED (IF ORDERED) FOR APPLICATION:
  - INSIDE- UNDER ROOF PANELS & BASE OF WALL PANELS OUTSIDE BETWEEN ROOF PANELS & RIDGE CAP BETWEEN WALL PANELS & EAVE/GABLE TRIM

ERECTION NOTE:
ALL BRACING, STRAPPING, & BRIDGING SHOWN AND PROVIDED BY M.B.S. FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE. IF ADDITIONAL BRACING IS REQUIRED FOR STABILITY DURING ERECTION, IT SHALL BE THE ERECTOR'S RESPONSIBILITY TO DETERMINE THE AMOUNT OF SUCH BRACING AND TO PROCURE AND INSTALL AS NEEDED.

1.9 ERECTION AND UNLOADING NOT BY G.W.B.

CUSTOMER AND DISALLOWED.

- ANY CLAIMS OR SHORTAGES BY BUYER MUST BE MADE TO M.B.S. WITHIN FIVE (5) WORKING DAYS AFTER DELIVERY, OR SUCH CLAIMS WILL BE CONSIDERED TO HAVE BEEN WAIVED BY THE
- CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)
  CLAIMS FOR CORRECTION OF ALLEGED MISTITS WILL BE DISALLOWED UNLESS M.B.S. SHALL
  HAVE RECEIVED PRIOR NOTICE THEREOF AND ALLOWED REASONABLE INSPECTION OF SUCH
  MISSITS. THE CORRECTION OF MINOR MISTITS BY THE USE OF DRIFT PINS TO DRAW THE
  COMPONENTS INTO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE
  REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND
  ARE NOT SUBJECT TO CLAIM. NO PART OF THE BUILDING MAY BE RETURNED FOR ALLEGED
  MISFITS WITHOUT THE PRIOR APPROVAL OF M.B.S.

### BUYER/END USE CUSTOMER RESPONSIBILITIES

- IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO OBTAIN APPROPRIATE APPROVALS AND SECURE NECESSARY PERMITS FROM CITY, COUNTY, STATE, OR FEDERAL AGENCIES AS REQUIRED, AND TO ADVISE/RELEASE M.B.S. TO FABRICATE UPON RECEIVING
- METAL BUILDING SUPPLIER (HEREAFTER REFERRED TO AS M.B.S.)
  STANDARD SPECIFICATIONS APPLY UNLESS STIPULATED OTHERWISE IN THE CONTRACT
  DOCUMENTS. M.B.S. DESIGN, FABRICATION, QUALITY CRITERIA, STANDARDS, PRACTICE,
  METHODS AND TOLERANCES SHALL GOVERN THE WORK WITH ANY OTHER INTERPRETATIONS
  TO THE CONTRARY NOTWITHSTANDING. IT IS UNDERSTOOD BY BOTH PARTIES THAT THE
  BUYER/END USE CUSTOMER IS RESPONSIBLE FOR CLARIFICATION OF INCLUSIONS OR
  EXCLUSIONS FROM THE ARCHITECTURAL PLANS AND/OR SPECIFICATIONS.
- IN CASE OF DISCREPANCIES BETWEEN M.B.S. STRUCTURAL STEEL PLANS AND PLANS FOR OTHER TRADES, M.B.S. PLANS SHALL GOVERN. (SECTION 3 AISC CODE OF STANDARD
- APPROVAL OF M.B.S. DRAWINGS AND CALCULATIONS INDICATE THE M.B.S. HAS CORRECTLY INTERPRETED AND APPLIED THE CONTRACT DOCUMENTS. THIS APPROVAL CONSTITUTES THE CONTRACTOR/OWNERS ACCEPTANCE OF THE M.B.S. DESIGN CONCEPTS, ASSUMPTIONS, AND LOADING. (SECTION 4 AISC CODE AND MBMA 3.3.3)
- ONCE THE BUYER/END USE CUSTOMER HAS SIGNED M.B.S. APPROVAL PACKAGE AND THE PROJECT IS RELEASED FOR FABRICATION, CHANGES SHALL BE BILLED TO THE BUYER/
  END USE CUSTOMER INCLUDING MATERIAL, ENGINEERING AND OTHER COSTS. AN ADDITIONAL FEE MAY BE CHARGED IF THE PROJECT MUST BE MOVED FROM THE FABRICATION AND

- THE BUTER/END USE CUSTOMERT IS RESPONSIBLE FOR OVERALL RESPONSIBLE FOR OVERALL PROJECT COORDINATION. ALL INTERFACE, COMPATIBILITY, AND DESIGN CONSIDERATIONS CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO BE CONSIDERED AND COORDINATED BY THE BUYER/END USE CUSTOMER. SPECIFIC DESIGN CRITERIAL CONCERNING THIS INTERFACE BETWEEN MATERIALS MUST BE FURNISHED BEFORE RELEASE FOR FABRICATION OR M.B.S. ASSUMPTIONS WILL GOVERN (AISC CODE OF STANDARD PRACTICE,
- 2.7 IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO INSURE THAT M.B.S. PLANS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT M.B.S. OR ITS DESIGN ENGINEERS ARE ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT. THESE DRAWINGS ARE SEALED ONLY TO CERTIFY THE DESIGN OF THE STRUCTURAL COMPONENTS FURNISHED BY M.B.S.
- 2.8 THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF STEEL IN ACCORDANCE WITH M.B.S. "FOR ERECTION" DRAWINGS ONLY. TEMPORARY SUPPORTS SUCH AS GUYS, BRACES, FALSEWORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION. OF THE FERECTION OPERATION SHALL BE DETERMINED, FURNISHED AND INSTALLED BY THE FRECTOR. NO ITEMS SHOULD BE PURCHASED FROM A PRELIMINARY SET OF DRAWINGS, INCLUDING ANCHOR BOLTS. USE ONLY FINAL "FOR ERECTION" DRAWINGS FOR THIS USE. (AISC CODE OF STANDARD DRACTOR LATERS FOR THIS USE.) PRACTICE, LATEST EDITION.)
- 2.9 METAL BUILDING SUPPLIER IS RESPONSIBLE FOR THE DESIGN OF THE ANCHOR BOLTS TO PERMIT THE TRANSFER OF FORCES BETWEEN THE BASE PLATE AND THE ANCHOR BOLT IN SHEAR, BEARING AND TENSION, BUT IT IS NOT RESPONSIBLE FOR THE TRANSFER OF ANCHOR BOLT FORCES TO THE CONCRETE OR THE ADEQUACY OF THE ANCHOR BOLT IN RELATIONTO THE CONCRETE. CONCRETE.

  UNLESS OTHERWISE NOTED PROVIDED IN THE ORDER DOCUMENTS, M.B.S. DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND CONSTRUCTIONOF THE FOUNDATION OR FOUNDATION EMBEDMENTS. THE END USE CUSTOMER SHOULD BE ASSURE HIMSELF THAT ADEQUATE PROVISIONS ARE MADE IN THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE. IT IS RECOMMENDED THAT THE ANCHORAGE AND FOUNDATION OF THE BUILDING BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER EXPERIENCED IN THE DESIGN OF SUCH STRUCTURES. (LATEST MBMA LOW RISE BUILDING SYSTEMS MANILA!)

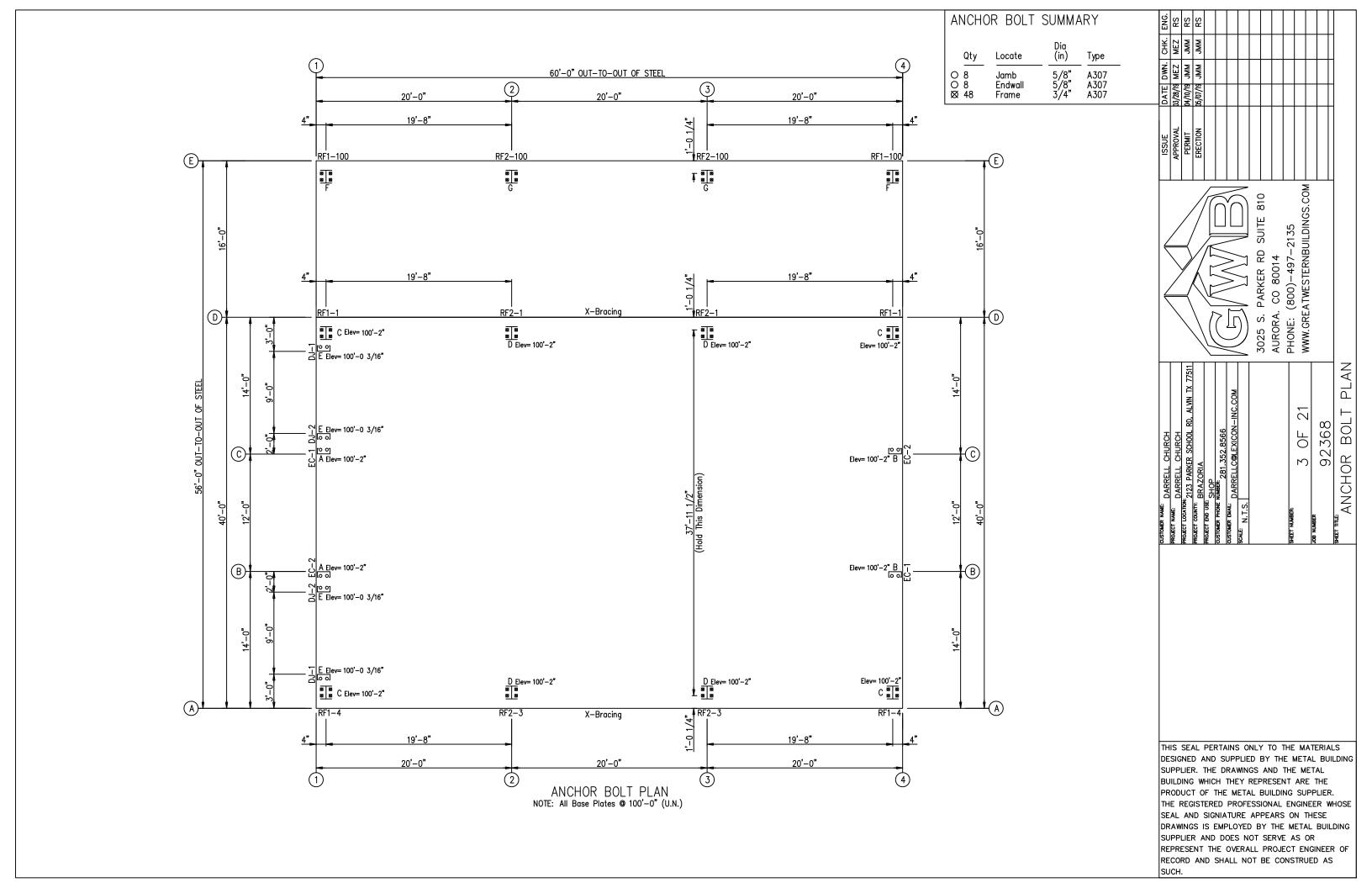
SYSTEMS MANUAL)

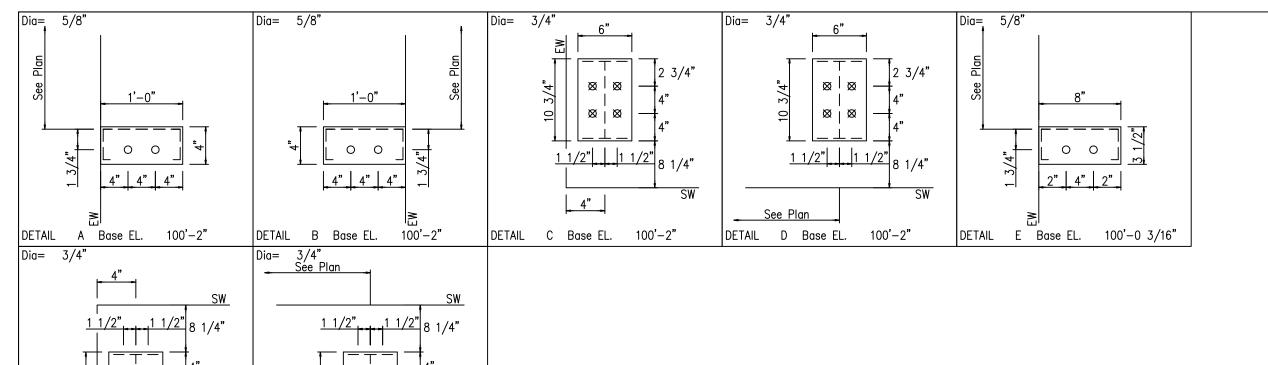
- 2.10 NORMAL ERECTION OPERATIONS INCLUDE THE CORRECTIONS OF MINOR MISFITS BY MODERATE AMOUNTS OF REAMING, CHIPPING, WELDING OR CUITING, AND THE DRAWING OF ELEMENTS INTO LINE THROUGH THE USE OF DRIFT PINS. ERRORS WHICH CANNOT BE CORRECTED BY THE FOREGOING MEANS OR WHICH REQUIRE MAJOR CHANGES IN MEMBER CONFIGURATION ARE TO BE REPORTED IMMEDIATELY TO M.B.S. BY THE BUYER/END USE CUSTOMER, TO ENABLE WHOEVER IS RESPONSIBLE EITHER TO CORRECT THE ERROR OR TO APPROVE THE MOST EFFICIENT AND ECONOMIC METHOD OF CORRECTON TO BE USED BY OTHERS. (AISIC CODE OF STANDARD PRACTICE LATEST EDITION)
- 2.11 NEITHER THE FABRICATOR NOR THE BUYER/END USE CUSTOMER WILL CUT, DRILL OR OTHERWISE ALTER HIS WORK, OR THE WORK OF OTHER TRADES, TO ACCOMMODATE OTHER TRADES, UNLESS SUCH WORK IS CLEARLY SPECIFIED IN THE CONTRACT DOCUMENTS. WHENEVER SUCH WORK IS SPECIFIED, THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR FURNISHING COMPLETE INFORMATION AS TO MATERIALS, SIZE, LOCATION AND NUMBER OF ALTERATIONS PRIOR TO PREPARATION OF SHOP DRAWNIGS. (AISC CODE OF STANDARD PRACTICE LATEST EDITION)
- 2.12 <u>Warning</u> in no case should galvalume steel panels be used in conjunction with lead or copper. Both lead and copper have harmful corrosive effects on the galvalume alloy coating when they are in contact with galvalume steel panels. Even Run-off from copper flashing, wiring, or tubing onto galvalume should be
- 2.13 SAFETY COMMITMENT METAL BUILDING SUPPLIER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE RECTOR ARE BEYOND THE CONTROL OF M.B.S. IT IS SETONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE. LOCAL, STATE, AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKERS SAFETY. MAKE CERTAIN ALL EMPOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING. EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES. DAILY MEETINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBER SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE, ARE RECOMMENDED.
- 2.14 ROOF DRAINAGE SYSTEMS (GUTTER, DOWNSPOUTS, ETC.) MUST BE FREE OF ANY OBSTRUCTION TO ENSURE SMOOTH OPERATION AT ANY GIVEN TIME.
- 2.15 IT IS RECOMMENDED BY FACTORY MUTAL (REFERENCE B2.44) THAT ROOFS BE CLEARED OF SNOW WHEN HALF OF THE MAXIMUM SNOW DEPTH IS REACHED. THE MAXIMUM SNOW DEPTH CAN BE ESTIMATED BASED ON THE DESIGN SNOW LOAD AND THE DENSITY OF SNOW AND/OR ICE BUILDUP. SSE TABLE BELOW.

ROOF SNOW LOAD (IN PSF)	EQUIVALENT SNOW HEIGHT AT ROOF (IN INCHES)	RECOMMENDED SNOW HEIGHT WHEN SNOW REMOVAL SHOULD START (IN INCHES)
20	16.60	8.30
25	17.25	8.62
30	17.90	8.95
35	18.55	9.28
40	19.20	9.60
45	19.85	9.92
50	20.50	10.25
55	21.15	10.58
60	21.80	10.90
65	22.45	11.22
70	23.10	11.55
75	23.75	11.88
80	24.40	12.20

FOR SNOW/ICE REMOVAL PROCEDURE, REFER TO METAL BUILDING SYSTEM MANUAL 2002 EDITION, SECTION A8.4, PAGE  $\rm XI-A8-2$ 

		BUILDING LOADS		RS G	2 S						
	THIS STRUCTURE HAS BEEN DESIGNED	IN ACCORDANCE WITH 1	THE FOLLOWING AS INDICATED:	CHK.	JAM JAM JAM						i
Ā	DESIGN LOADS: DESIGN CODE / WIND CODE	BldgA: : IBC-15	<u>BldgB:</u> : IBC-15	E DWN.	WWC SI						l
	BUILDING RISK CATEGORY ENCLOSURE DEAD LOAD (PSF) COLLATERAL LOAD (PSF)	:II — Normal :Closed :2.00 :1.00	:    — Normal : Partially Enclosed : 2.00	DATE 03/28/19	04/10/					$\frac{1}{1}$	
:	WIND LOAD  ULTIMATE WIND SPEED, (VULT) (MPH)  WIND EXPOSURE	:145.0 :C	:1.00 :145.0 :C	ISSUE	ERECTION						
Y R R.	INTERNAL PRESSURE COEFFICIENT, GCpi WALL PANEL DESIGN WIND PRESSURE (PSF) LIVE LOAD	: 0.18/-0.18	: 0.55/-0.55 : 56.7/-60.2						Ţ		ı
т	PRIMARY FRAMING (PSF) TRIB. AREA REDUCTION SECONDARY FRAMING (PSF)	: 20.00 : No : 20.00	: 20.00 : No : 20.00		//(		810		GS.CON		
D	SNOW LOAD GROUND SNOW LOAD, Pg (PSF) ROOF SNOW LOAD, Pf (PSF)	: 0.00	: 0.00		/   //		SUITE	-2135	SUILDIN		
IN	SLOPED ROOF SNOW LOAD, Ps (PSF) SNOW EXPOSURE FACTOR, Ce SNOW IMPORTANCE FACTOR, Is	: 0.00 : 1.00 : 1.00	: 0.00 : 0.00 : 1.00 : 1.00		) [	$\gtrsim$	V LL LL PARKER RD	CO 80014 (800)-497-	STERNE		
	THERMAL FACTOR, Ct SLOPED FACTOR, Cs SEISMIC LOAD	:1.00 :1.00	: 1.00 : 1.00 : 1.00		//				(300) EATWE		<u> </u>
E	SEISMIC IMPORTANCE FACTOR, IE SITE CLASS MAPPED SPECTRAL RESPONSE ACCELERATION				//(		3025 S.	AURORA. PHONE: (	WWW.GREATWESTERNBUILDINGS.COM		COVERSHEE
E S	SPECTRAL RESPONSE COEFFICIENTS SEISMIC DESIGN CATEGORY BASIC FORCE RESISTING SYSTEMS USED	: Sds = 0.075 : Sd1 = 0.059 : A : STEEL SYSTEM NOT SPECIFIC	CALLY			${\prod}$	<u>_                                    </u>	▼ <u>0</u>	. <		VER
	TOTAL DESIGN BASE SHEAR, V (KIPS)	DETAILED FOR RESISTANCE :RIGID FRAMES (OMF) :BRACED FRAMES (OCBF/OMI :LONGITUDINAL = 0.53	-		ALVIN TX 77511	IC.COM			21		00 0
	RESPONSE MODIFICATION FACTORS, R	: TRANSVERSE = 0.44 : RIGID FRAMES = 3.50	: 0.00 : 0.12 Ω= 5.00		SCHOOL RD,	NUMBER: 281.352.8566 DARRELLC@LEXICON—INC.COM			OF 2	368	INF(
>	,	:SW X-BRACING = 5.00	$\Omega$ = 5.00	DARRELL CHURCH DARRELL CHURCH	ORIA	281.352.8566 ELLC@LEXICON			2	92	JILDING
ER IS	SEISMIC RESPONSE COEFFICIENTS, Cs	:RIGID FRAMES = 0.0213 :SW X-BRACING = 0.0149		ME: DARRELL	2123 PARKER S PROJECT COUNTY: BRAZORIA PROJECT END USE:	SHOP TONE NUMBER: MAIL: DARR	N.T.S.	Æ			BUILE
	ANALYSIS PROCEDURE USED OTHER LOADS/REQUIREMENTS	:EQUIVALENT LATERAL FORCE	PROCEDURE	CUSTOMER NAME: PROJECT NAME: PROJECT LOCATION	PROJECT COL	CUSTOMER PHONE	Z Z	SHEET NUMBI		OUD NUMBER	SHEET IIILE:
	BUILDING DESCRIPTION: BIdgA: WIDTH (FT) : 40 LENGTH (FT) : 60 EAVE HEIGHT AT BSW (FT): 18.17 EAVE HEIGHT AT FSW (FT): 18.17 ROOF SLOPE AT BSW : 2.0:12 ROOF SLOPE AT FSW : 2.0:12	BldgB: : 16 : 60 : 15.5 : 18.17 : 2.0:12 :									
	BAY SPACING (FT) : 3 at 20  COVERING AND TRIMS:  ROOF PANELS & TRIMS	:3 at 20									
	PANEL TYPE : 26 Ga. PBR PANEL COLOR : Rustic Red TRIM COLORS	: 26 Ga. PBR : Rustic Red									
	GABLE/EAVE : Rustic Red  WALL PANELS & TRIMS  PANEL TYPE : 26 Ga. PBR  PANEL COLOR	:Rustic Red									
	PANEL COLOR :Light Stone TRIM COLORS  CORNER :Rustic Red OPENING :Rustic Red BASE :Rustic Red			DESIGN SUPPLI	ED AN ER. TH	D SUPP IE DRAV	ONLY T LIED BY JINGS AN Y REPRE	THE I	METAL E MET	_ BUIL 「AL	
	WAINSCOT PANELS & TRIMS  PANEL TYPE : 26 Ga. PBR  PANEL COLOR : Rustic Red  TRIM COLORS : Rustic Red  INSULATION  ROOF INSULATION : 3" (R-10) WMP-VR			PRODU THE RE SEAL A DRAWIN	CT OF EGISTEF AND SIG IGS IS	THE ME RED PRO GNIATUR EMPLO	TAL BUI DFESSION RE APPE YED BY NOT SE	LDING IAL EN ARS O THE M	SUPF NGINEI ON THI IETAL	PLIER. ER WH ESE BUILD	HOSE
	WALL INSULATION : 3" (R-10) WMP-VR						RALL PR NOT BE				OF





## NOTE:

DETAIL

10 3/4"

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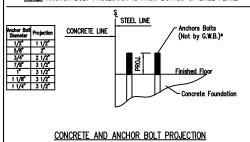
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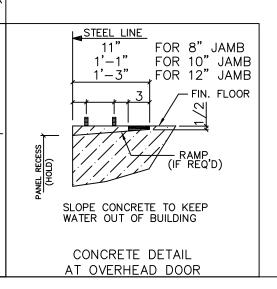
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. G.W.B. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ANCHOR BOLT DIAMETERS HAVE BEEN DESIGNED BY THE METAL BUILDING ENGINEER BASED ON AISC METHOD WITH COMBINED SHEAR AND TENSION.

DEVELOPMENT, EMBEDMENT AND HOOK LENGTH OF ANCHOR BOLTS IN THE CONCRETE ARE DESIGN RESPONSIBILITY OF OTHERS. ALSO DESIGN OF SHEAR ANGLES, TENSION PLATES, HAIRPINS, AND ANY OTHER EMBEDDED MATERIAL IN THE CONCRETE SHALL BE DESIGNED AND PROVIDED BY OTHERS.

NOTE: ANCHOR BOLT PROJECTION IS FROM BOTTOM OF BASE PLATE.





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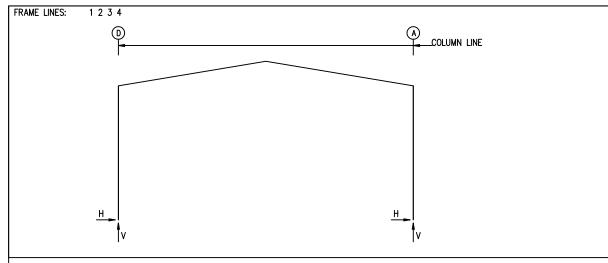
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THIS SEAL PERTAINS ONLY TO THE MATERIALS DESIGNED AND SUPPLIED BY THE METAL BUILDING SUPPLIER. THE DRAWINGS AND THE METAL BUILDING WHICH THEY REPRESENT ARE THE PRODUCT OF THE METAL BUILDING SUPPLIER. THE REGISTERED PROFESSIONAL ENGINEER WHOSE SEAL AND SIGNIATURE APPEARS ON THESE DRAWINGS IS EMPLOYED BY THE METAL BUILDING SUPPLIER AND DOES NOT SERVE AS OR REPRESENT THE OVERALL PROJECT ENGINEER OF RECORD AND SHALL NOT BE CONSTRUED AS SUCH.



			Colu	umn_Reac	tions(k	) .							
Frm Line	Col Line	Load Id	Hmax H	V Vmax	Loàd Id	´ Hmin H	V Vmin	Boli Qty	t(in) Dia	Base Width	e_Plate(in) Length -	Thick	Grout (in)
1*	D	2 1	2.6 1.3	3.6 7.0	6 4	-3.4 -1.6	-3.7 -5.6	4	0.750	6.000	10.75	0.375	2.0
1*	Α	5 1	3.3 -1.3	-5.2 5.1	3 5	-2.2 3.3	4.3 -5.2	4	0.750	6.000	10.75	0.375	2.0

				umn_Reac	tions(k								
Frm Line	Col Line	Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bol Qty	t(in) Dia	Base Width	e_Plate(in) Length 	Thick	Grout (in)
2*	D	2 1	5.3 3.1	6.5 13.3	6 7	-6.9 3.8	-7.4 -12.3	4	0.750	6.000	10.75	0.375	2.0
2*	Α	5 1	6.9 -3.2	-10.4 9.6	3 8	-4.6 1.7	7.9 -12.0	4	0.750	6.000	10.75	0.375	2.0

NOTES	FOR REACTIONS	
	ding reactions are based on following building data:  Width (ft) Length (ft) Eave Height (ft) Roof Slope (rise/12) Dead Load (psf ) Collateral Load (psf ) Live Load (psf ) Wind Speed (mph ) Wind Code Exposure Closed/Open Importance Wind Importance Seismic Seismic Coeff (Fa*Ss)	= 40.0 = 60.0 = 18.2/18.2 = 2.0/2.0 = 2.0 = 1.0 = 20.0 = 145.0 = IBC-15 = C = C = 1.00 = 1.00 = A = 0.11
ID	Description	
1 2 3 4 5 6 7 8 9 10	Dead+Collateral+Live Dead+Collateral+0.75Live+0.45W Dead+Collateral+0.75Live+0.45W 0.6Dead+0.6Wind_Left1 0.6Dead+0.6Wind_Right1 0.6Dead+0.6Wind_Left2 0.6Dead+0.6Wind_Long1L 0.6Dead+0.6Wind_Long2L 0.6Dead+0.6Wind_Pressure+0.6Wind_Pressure+0.6Wind_Right2+0.6Wind_Right2+0.6Wind_Pressure+0.6Wind_Right2+0.6Win	/ind_Left2 id_Suction Wind_Long2L

RIGI	D FRAM	νE:	BAS	IC COLUM	IN REACT	IONS (k	)						
Frame Line 1* 1* 2* 2*	e Column Line D A D A	Horiz 0.2 -0.2 0.4 -0.4	-Dead Vert 1.1 0.9 1.7 1.3		oteral- Vert 0.3 0.2 0.6 0.4	Horiz 1.1 -1.1 2.6 -2.7	-Live Vert 5.6 4.0 10.9 7.9	Wind Horiz -2.8 -0.7 -6.2 -0.7	Left1- Vert -10.5 -4.9 -20.4 -9.5	-Wind_ Horiz 3.6 5.7 6.3 11.8	Right1- Vert -4.5 -9.5 -8.9 -18.6	Wind Horiz -5.9 -2.6 -11.9 -4.5	J_Left2- Vert -7.2 0.3 -14.1 0.6
Frame Line 1* 1* 2* 2*	e Column Line D A D A	-Wind_ Horiz 0.5 3.8 0.6 8.0	Right2- Vert -1.3 -4.3 -2.6 -8.5	Wind Horiz 3.4 1.8 6.0 4.0	I_Long1- Vert -7.1 -7.5 -22.2 -19.6	Wind Horiz 2.9 1.3 5.1 3.2	I_Long2- Vert -6.2 -8.5 -20.4 -21.4	-Seism Horiz 0.0 0.0 -0.1 -0.1	ic_Left Vert 0.0 0.0 -0.1 0.1	Seismic Horiz 0.0 0.0 0.1 0.1	Right Vert 0.0 0.0 0.1 -0.1	-Seism Horiz 0.0 0.0 0.0 0.0	ic_Long Vert 0.0 0.0 -0.4 -0.1
1* 2*	Frame lir Frame lir		1 4 2 3										

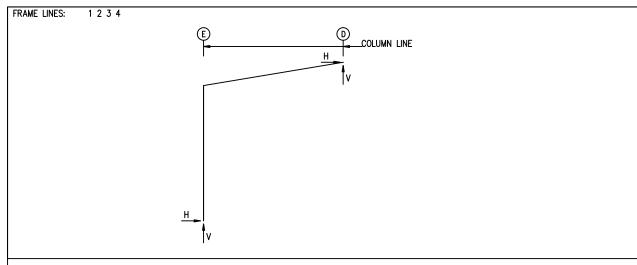
ENDW	ALL CO	LUMN:		BASIC C	DLUMN F	REACTIONS	s (k )						
Frm Line 1 1 4	Col De Line Ve C 0.1 B 0.1 B 0.1 C 0.1	ead f ert f 1 – 1 –	Wind Press Horz 4.3 4.3 4.3	Wind Suct Horz 4.7 4.7 4.7			,						
ENDW	ALL CO	LUMN:		MAXIMUM	REACTION	ONS, ANCH	HOR BOLTS,	& B	ASE PLATE	ES			
Frm Line		Load Id	Co Hmax H	lumn_Reac V Vmax 	tions(k Load Id	Hmin H	V Vmin	Bol Qty	t(in) Dia	Base Width	e_Plate(in) Length	Thick	Grout (in)
			Hmax	٧	Load	Hmin		Bol Qty 2	t(in) Dia 0.625		e_Plate(in) Length - 12.00	- Thick - 0.375	Grout (in) 2.0
	Line	_ <u>ld</u> 9	Hmax H 2.8	V Vmax 0.1	Load Id	Hmin H	Vmin	Qty	Dia	Width	Length ´		_ (in)
	Line C	_ Id 9 11 9	2.8 2.8 2.8 2.8	V Vmax 0.1 0.1 0.1	Load Id ——————————————————————————————————	Hmin H -2.6	0.1	Qty 2	0.625	4.000	Length 12.00	0.375	(in) 

—— Wc	— ااد	- Col	±	Reacti	ions(k ) - —Sei	smic -	Panel_ - (lb	_Shear /ft)	
.ос	Line	Line	Horz	Vert	Horz	Vert	Wind	Seis	Note
_EW _SW	1		• •		•				(h)
_SW R_EW	A 4 D	2,3	6.0	4.9	0.1	0.1			(h)
SW	Ď	3,2	10.2	8.3	0.4	0.3			(11)

ANCHO	OR BOLT	SUMMA	\RY	
Qty	Locate	Dia (in)	Туре	
O 8 O 8 Ø 32	Jamb Endwall Frame	5/8" 5/8" 3/4"	A307 A307 A307	

CUSTOMER NAME: DARRELL CHURCH	<b>←</b>	ISSUE	DATE DWN. CHK. ENG.	CHK.	ENG.
PROJECT NAME: DARRELL CHURCH		APPROVAL	03/28/19 MEZ MEZ	MEZ	RS
		PERMIT	MML 81/01/40	MMC	RS
PROJECT COUNTY: BRAZORIA		ERECTION	D5/07/19 JMM	MM	RS
PROJECT END USE: SHOP					
OUSTOMER EMAIL: DARRELLC@LEXICON—INC.COM					
SCALE: N.T.S.					
	018 TTIII AG GTYGAG 3 3005				
	SUZS S. PARNER RD SUITE OLD				
	AURORA. CO 80014				
SHEET NUMBER:	PHONE: (800)-497-2135				
5 OF 21	WWW CDEATWESTERNING III IN WINCO				
JOB NUMBER	WWW.GREALWESTERNBOILDINGS.COM				
92368					
ANCHOR BOLT REACTIONS FOR BLDG A	TIONS FOR BLDG A				

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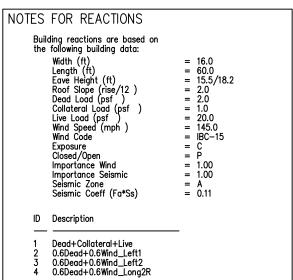
_				ımn_Reac						_	5 (c.)		
Frm Line	Col Line	Load Id	Hmax H	V Vmax	Load Id — —	Hmin H	V Vmin	Bol Qty	t(in) Dia	Base Width	e_Plate(in) Length 	Thick	Grout (in)
1*	Ε	4 1	1.7 0.0	-2.7 2.3	3 4	-1.7 1.7	0.3 -2.7	4	0.750	6.000	10.75	0.375	0.0
1*	D	4 1	2.8 0.0	-1.7 1.8	3 2	-1.9 1.1	-0.4 -2.0						

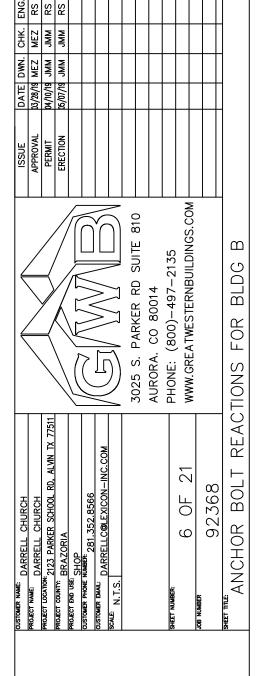
RIGID	FRAME:		MAXIMUM	REACTION	S. ANCI	HOR BOLT	S, & BASE	E PLATI	 Es				
Frm Line	Col	Load Id		umn_React V Vmax	-		V Vmin		t(in) Dia	Base Width	e_Plate(in) Length	Thick	Grout (in)
2*	Ε	4 1	3.2 0.1	-5.3 4.2	3 4	-3.3 3.2	0.5 -5.3	4	0.750	6.000	10.75	0.375	0.0
2*	D	<b>4</b> 1	5.5 -0.1	-3.3 3.5	3 2	-3.7 2.1	-0.9 -3.9						
2*	Frame lin	es:	2 3										

BUILDING BRACING REACTIONS	
Wall Col	Note
L_EW 1 F_SW D R_EW 4 B_SW E Torsional Bracing Used	(h) (e) (h)
(e)Bracing loads must be applied to supporting building (h)Rigid frame at endwall	

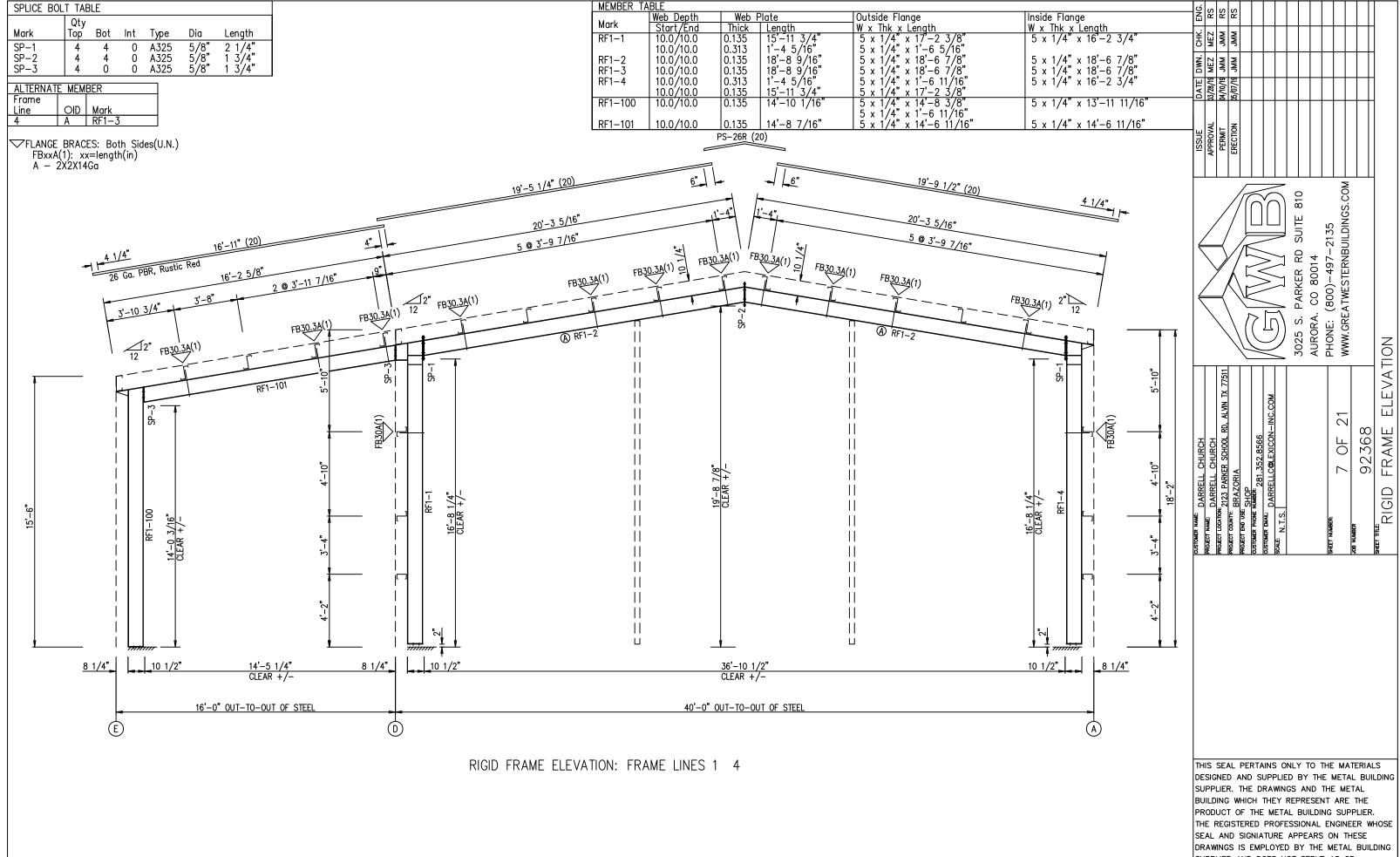
ANCHO	OR BOLT	SUMMA	ARY
Qty	Locate	Dia (in)	Туре
<u>⊠</u> 16	Frame	3/4"	A307

RIGII	FRAN	ΛE:	BAS	IC COLUM	IN REACT	IONS (k	)								
Frame Line 1* 1* 2* 2*	Column Line E D E D	Horiz 0.0 0.0 0.0 0.0	-Dead Vert 0.5 0.2 0.6 0.4		oteral- Vert 0.1 0.1 0.2 0.1	Horiz 0.0 0.0 0.1 -0.1	-Live Vert 1.7 1.5 3.4 3.0	Wind Horiz 0.2 1.8 0.5 3.4	Left1- Vert -4.4 -3.5 -8.6 -6.8	-Wind_ Horiz 2.4 4.0 4.8 7.8	Right1- Vert -3.8 -2.2 -7.4 -4.4	Wind Horiz -2.8 -3.1 -5.5 -6.1	_Left2- Vert 0.1 -1.0 0.1 -1.9		
Frame Line 1* 1* 2* 2*	Column Line E D E D	-Wind_ Horiz -0.6 -0.9 -1.2 -1.8	Right2- Vert 0.6 0.3 1.3 0.6	Wind Horiz 2.8 4.7 5.4 9.2	I_Long1- Vert -4.9 -3.0 -9.5 -5.8	Wind Horiz 2.8 4.7 5.4 9.2	I_Long2- Vert -4.9 -3.0 -9.5 -5.8	-Seism Horiz 0.0 0.0 0.0 0.0	ic_Left Vert 0.0 0.0 0.0 0.0	Seismic Horiz 0.0 0.0 0.0 0.0	_Right Vert 0.0 0.0 0.0 0.0				
1* 2*	Frame lin Frame lin		1 4 2 3												

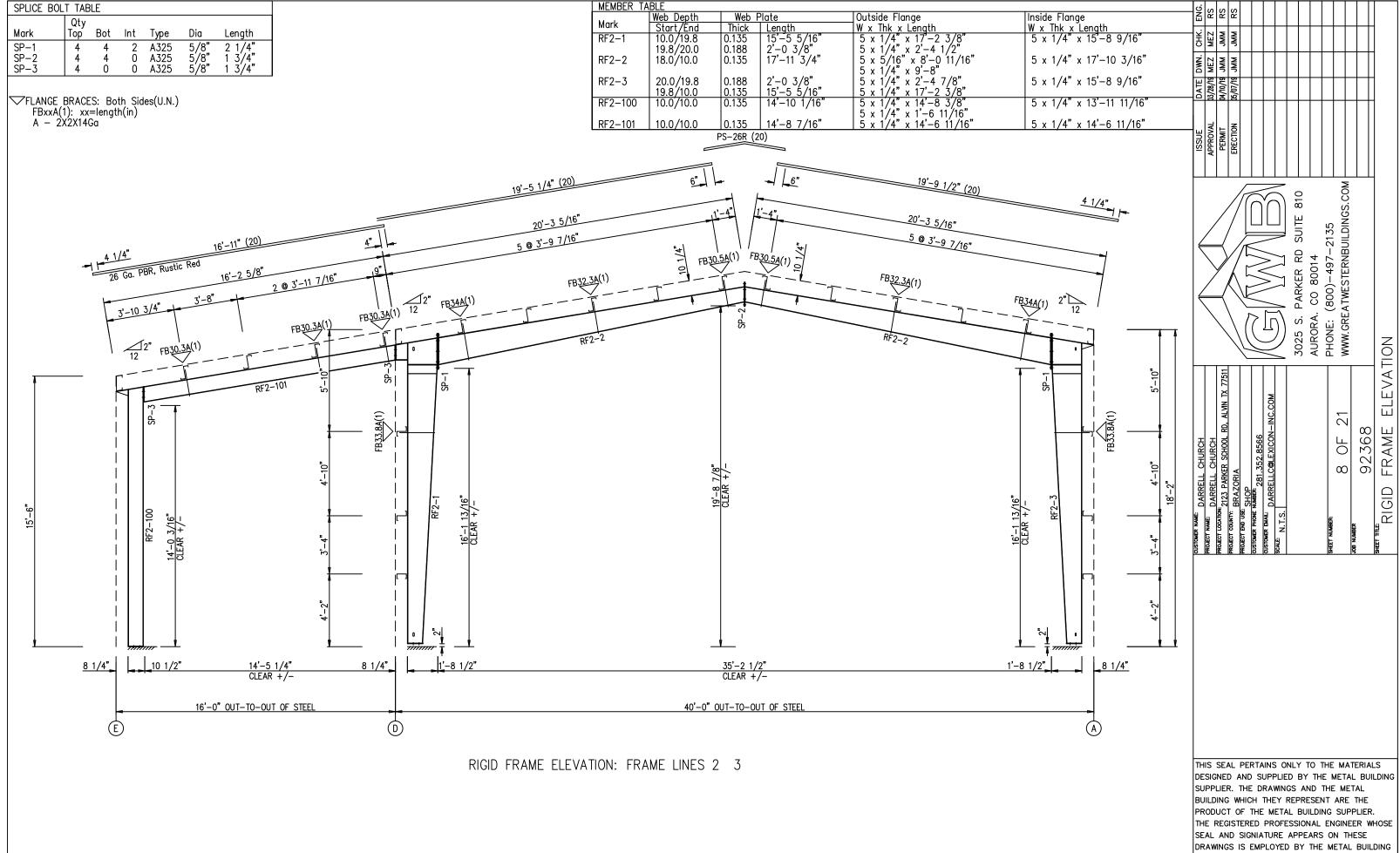




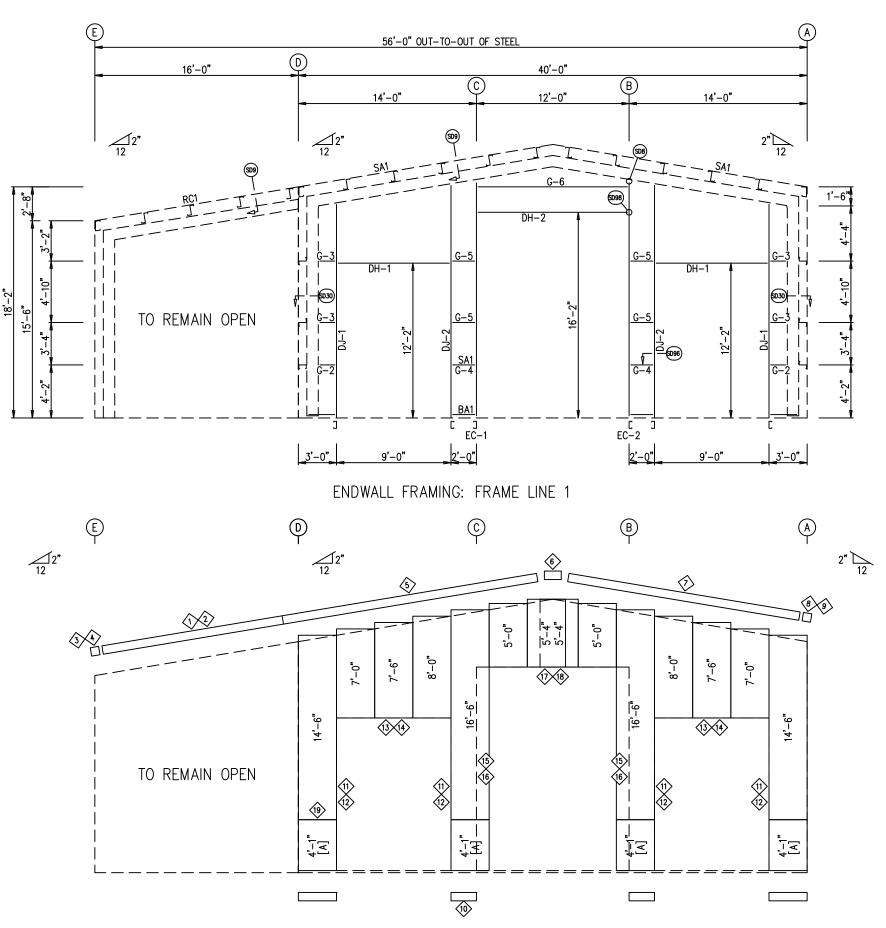
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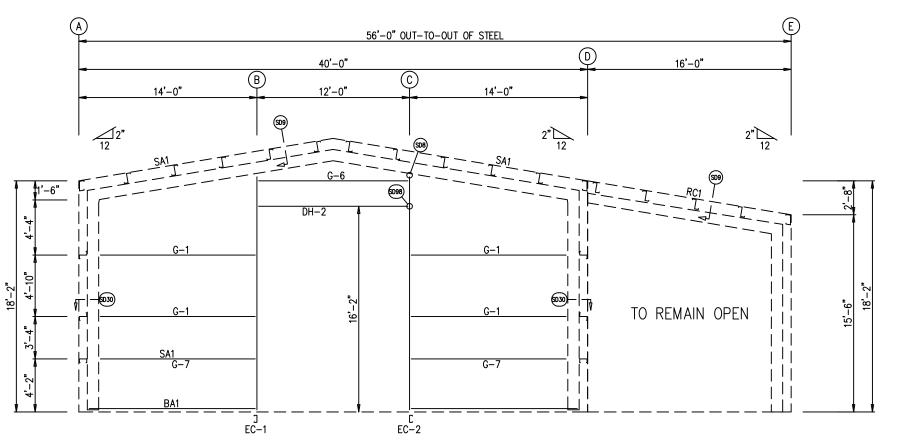
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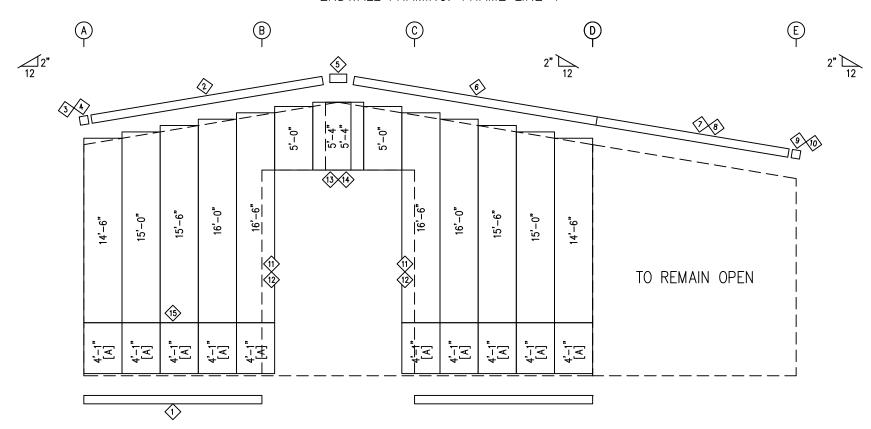
ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 Ga. PBR — Light Stone
[A] PANELS: 26 Ga. PBR — Rustic Red

	BI Co	FRAM ⇒ID  1  2  3  4  5  6  7  8  9  10  11  12  13  14  15  16  17  18  19  BO
	AME LIN CCATION dgA olumns/F mb/Raft MEMBEF FRAME QUAN BIdgA 1 1 2 2 2 1 2 4 2 4 1	TABLE ME LINE QUAN 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
	Rafter er R TABLE	1 MARK FL-21 FL-365 FL-21L FL-328R FL-21 FL-23 FL-21R FL-328L FL-60 FL-55 FL-55 FL-48 FL-55 FL-48 FL-55 FL-301 FL-301 FL-301 FL-237
	QUAN  2 2 2  PART  12X35C12 12X35C12 8X35C14 8X35C14 8X25C16 12X35C16 8X25C16 8X25Z16 8X25Z16 8X25Z16 8X25Z16 8X25Z16	LENGTH 13'-5" 16'-3" 11'-2" 9 1/2" 13'-5" 1'-4" 10'-4" 11'-2" 9 1/2" 12'-2" 12'-2" 12'-3" 9'-4" 9'-4" 16'-2" 12'-4" 11'-4" 11'-2"
	TYPE DIA  A325 5/8  A325 5/8    LENGTI 2 18'-6 2 18'-6 16'-10 18'-4 9'-0" 1 1'-0 1 1'-0 1 1'-3 1 1'-3 1 11'-11	
		TAIL 225 225 885 113 335 135 135 151 151 152 152 152 151 152 152 152 15
DES SUF BUII PRO THE SEA DRA SUF REF	CUSTOMER NAME: DARRELL CHURCH	DATE DWN. CHK.
EIGNED PPLIER LDING DDUCT REGI AL ANI AWING PPLIER PRESEI	PROJECT CONTINUE. 2123 PARKER SCHOOL RD. ALVIN TX 77511	03/28/19 MEZ 04/10/19 JMM
AND HAND STER D SIG	PROJECT END USE: SHOP	ERECTION D6/07/19 JMM RS
D SUFE DRACH THE I	CUSTOMER PHONE NUMBER 281,352,8566 CUSTOMER BANU:	
PPLIE AWING HEY F META ROFE URE OYEC ES NO	SCALE N.T.S.	
D BY GS AI REPRI L BU GSSION APPE DBY CL PI	3025 S. PARKER RD	ID SUITE 810
THE ND TO ESENI ILDIN NAL I ARS THE ERVE ROJEO		
HE MATE METALIS SUPPLEMENTE OF LENGUES OF LE	SHEET NUMBER: 9 OF 21 WWW.GREATWESTERNBUILDINGS.COM	7-2135 NBUILDINGS.COM
L BUII TAL THE PLIER. ER WI ESE BUILI R	JOS NUMBER 92368	
HOSE DING	SHEETING & SHEETING FO	FOR BLDGS A & B



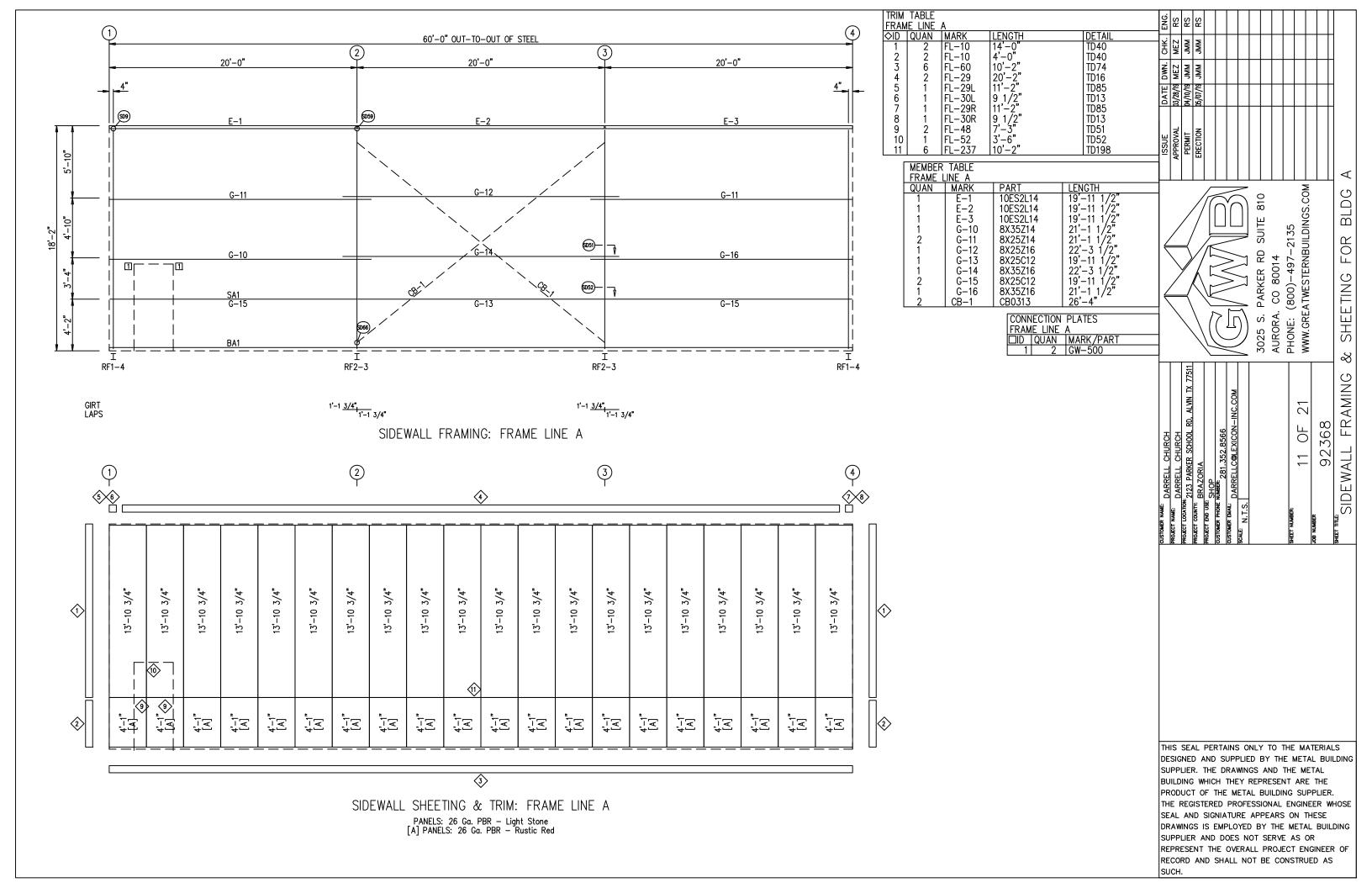
# ENDWALL FRAMING: FRAME LINE 4

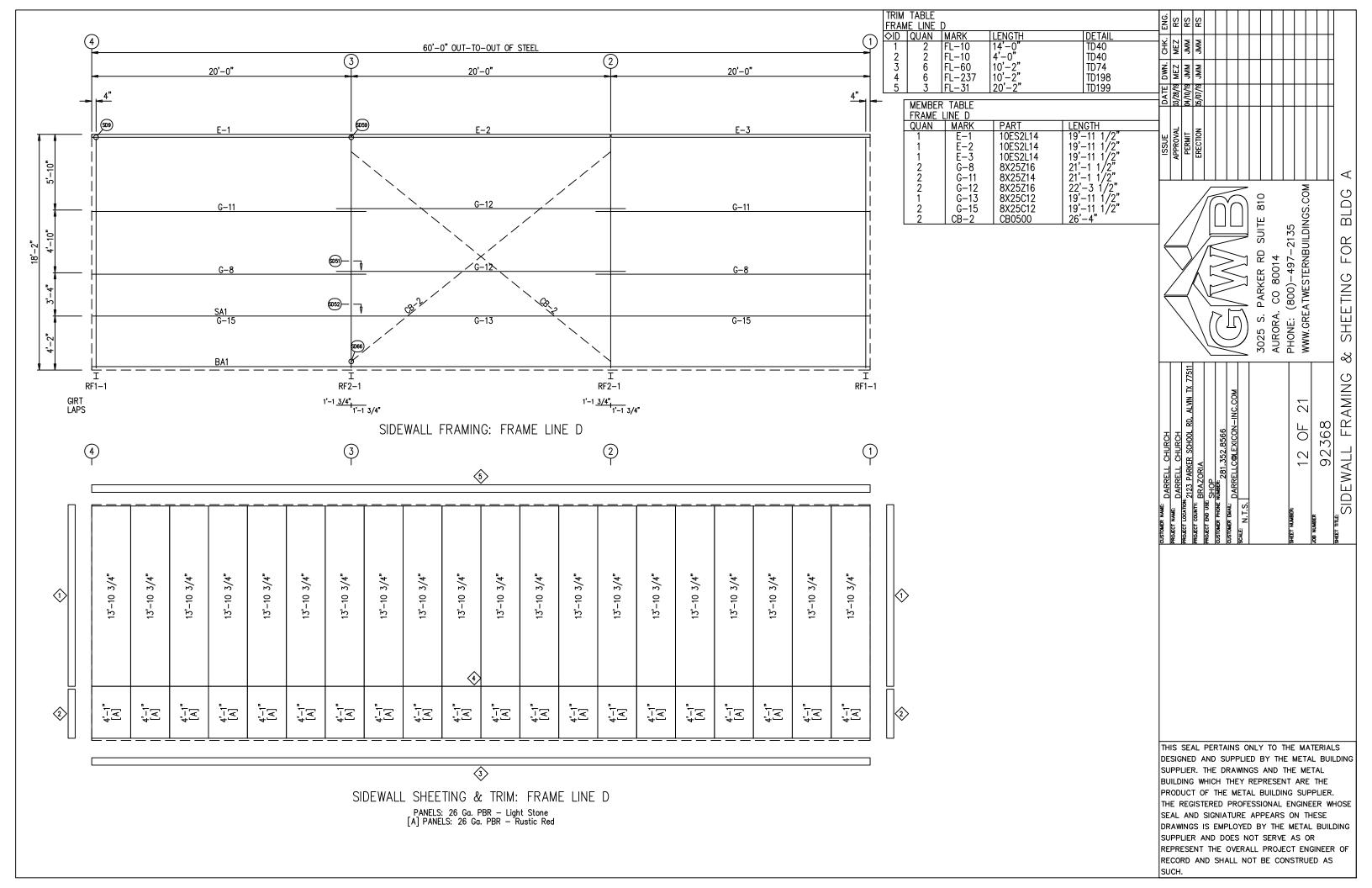


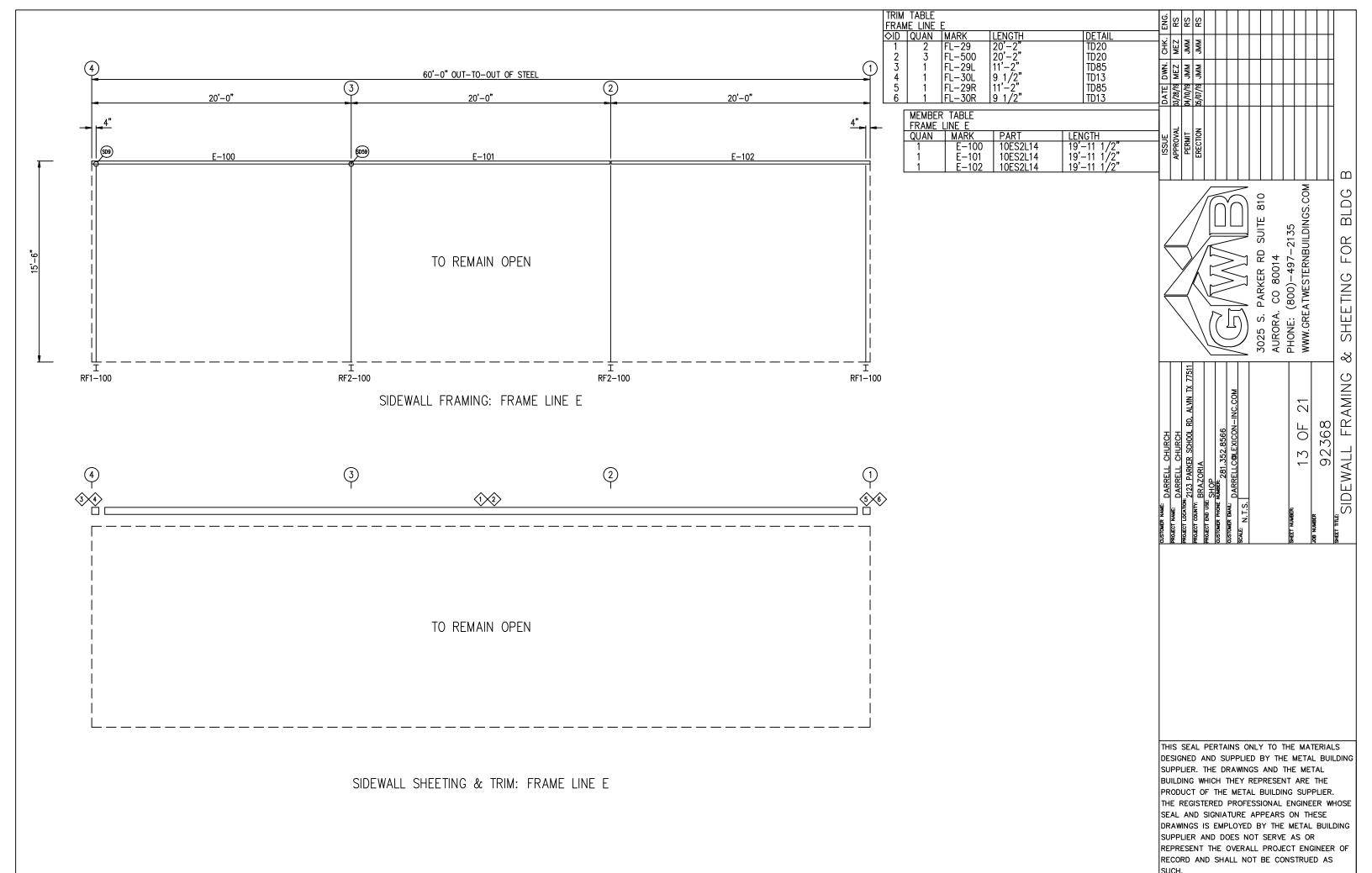
ENDWALL SHEETING & TRIM: FRAME LINE 4

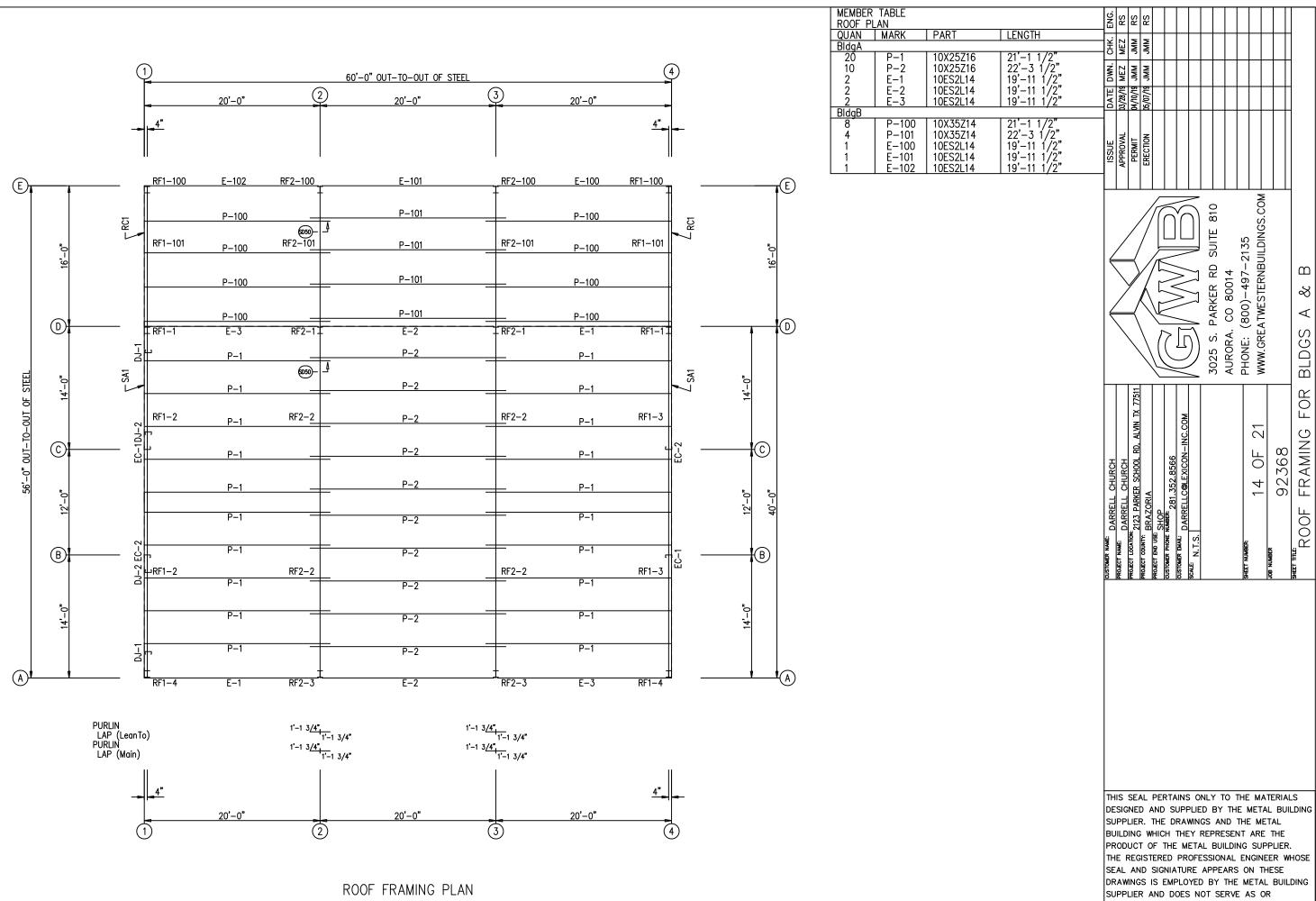
PANELS: 26 Ga. PBR - Light Stone
[A] PANELS: 26 Ga. PBR - Rustic Red

TRIM TABLE FRAME LINE 4 ◇ID QUAN   MARK   LENGTH   DETAIL	R R R R R R R R R R R R R R R R R R R
OID         QUAN         MARK         LENGTH         DETAIL           1         3         FL−60         10′−2″         TD74           2         1         FL−21         10′−4″         TD35	OHK.
3   1   FL-21L   11'-2"   TD85   4   1   FL-328R   9 1/2"   TD13	DWN.
5 1 FL-23 1'-4" 6 1 FL-21 13'-5" TD35 7 1 FL-21 13'-5" TD25 8 1 FL-365 16'-3" TD25	DATE D3/28/19 D4/10/19 D5/07/19 D5/07/19
7 1 FL-21 13'-5" TD25 8 1 FL-365 16'-3" TD25 9 1 FL-21R 11'-2" TD85 10 1 FL-328L 9 1/2" TD13 11 2 FL-301 16'-2" TD51	ISSUE APPROVAL PERMIT ERECTION APPROVAL
10 1 FL-328L 9 1/2" TD13 11 2 FL-301 16'-2" TD51 12 2 FL-48 16'-3" TD51 13 1 FL-301 12'-4" TD52 14 1 FL-52 12'-4" TD52 15 3 FL-237 10'-2" TD198	A A S
14 1 FL-52 12'-4" TD52 15 3 FL-237 10'-2" TD198	S S INGS. COM
FRAME LINE 4  LOCATION QUAN TYPE DIA LENGTH BIdgA	3025 S. PARKER RD SUITE 810 AURORA. CO 80014 PHONE: (800)—497—2135 WWW.GREATWESTERNBUILDINGS.COM
Columns/Rafter 2 A325 5/8" 1 1/4"  MEMBER TABLE	RD St. 97-21. RNBUIL
FRAME LINE 4 QUAN   MARK   PART   LENGTH	PARKER RD SUIT CO 80014 (800)-497-2135 ATWESTERNBUILDIN
1 EC-1 12X35C12 18'-6 3/16" 1 EC-2 12X35C12 18'-6 3/16" 1 DH-2 12X35C14 12'-0"	D25 S. PARKER URORA. CO 80 HONE: (800)-4 WW.GREATWESTE
1   EC-1   12X35C12   18'-6 3/16" 1   EC-2   12X35C12   18'-6 3/16" 1   DH-2   12X35C14   12'-0" 4   G-1   8X25Z16   12'-0 7/8" 1   G-6   8X25Z16   11'-11 1/2" 2   G-7   8X25C16   12'-0 7/8"	3025 S. AURORA. PHONE: WWW.GRE
2   0 /   0/23010   12 0 //0	x 77511
	서     1   1   1   1   1   1   1
	CHURCH CHURCH CHURCH A A 352.8566 C@LXICON- C@LXICON- COLXICON- CO
	DARRELL CHURCH 2123 PARKER SCHOOL 2123 PARKER SCHOOL 2123 PARKER SCHOOL 2125 PARKER SCHOO
	IN ANNE: DAR NAME: DAR NAME: DAR NAME: DAR NAME: DAR NAME: BRO USE: SHO NAME: DAR NAME
	CHARTONER NAME DARRELL CHURCH PROJECT NAME 123 PARKER SCHOOL   PROJECT COUNTY BRAZORIA PROJECT END USE SHOP CUSTOMER PHONE NAMER 281.352.8566 CUSTOMER BIANL DARRELLC@LEXICON SCALE N.T.S.  10 OF USE NUMBER  SHEET NUMBER  9236 SHEET NUMBER  9256 SHEET NUMBER
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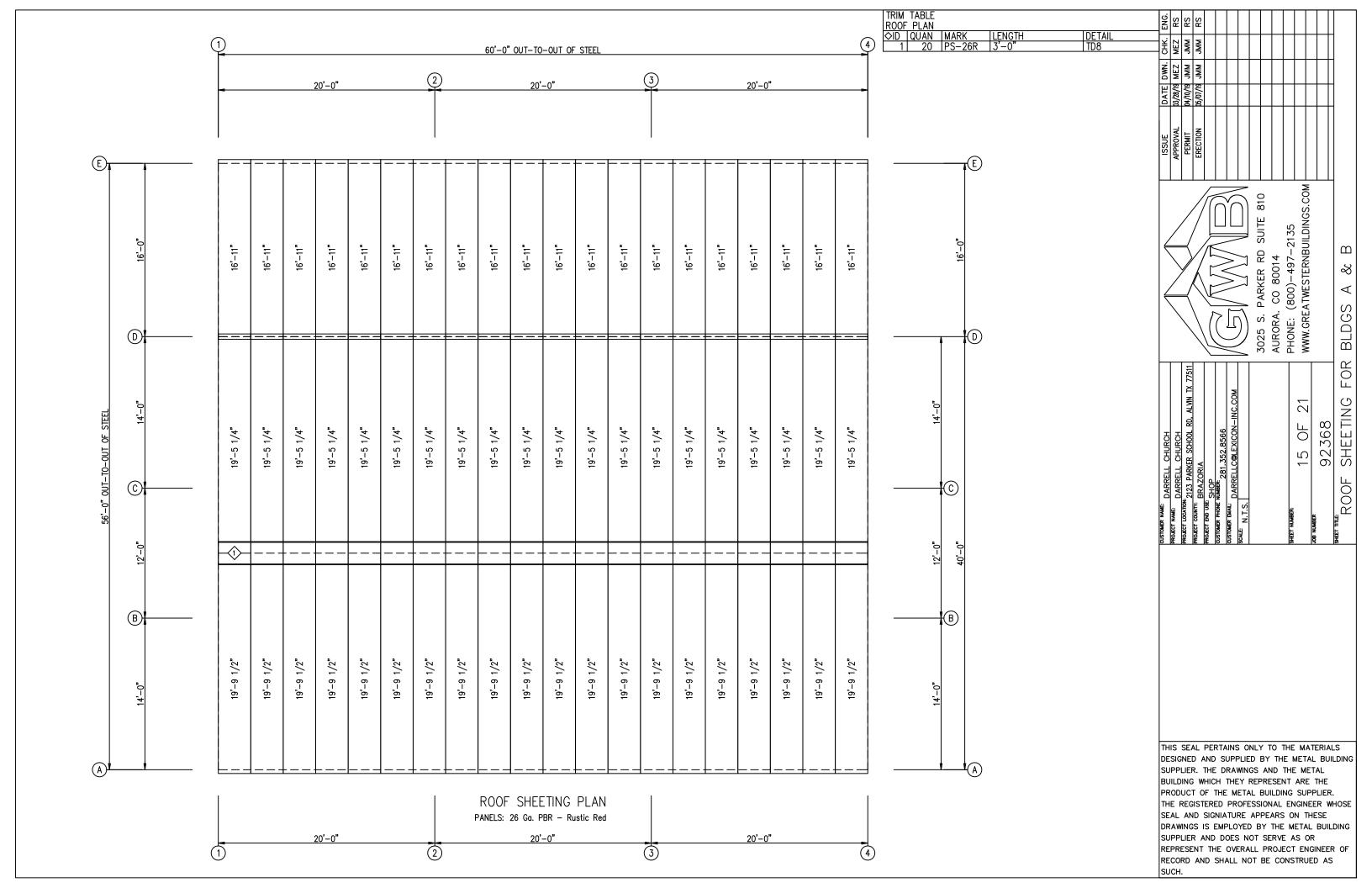


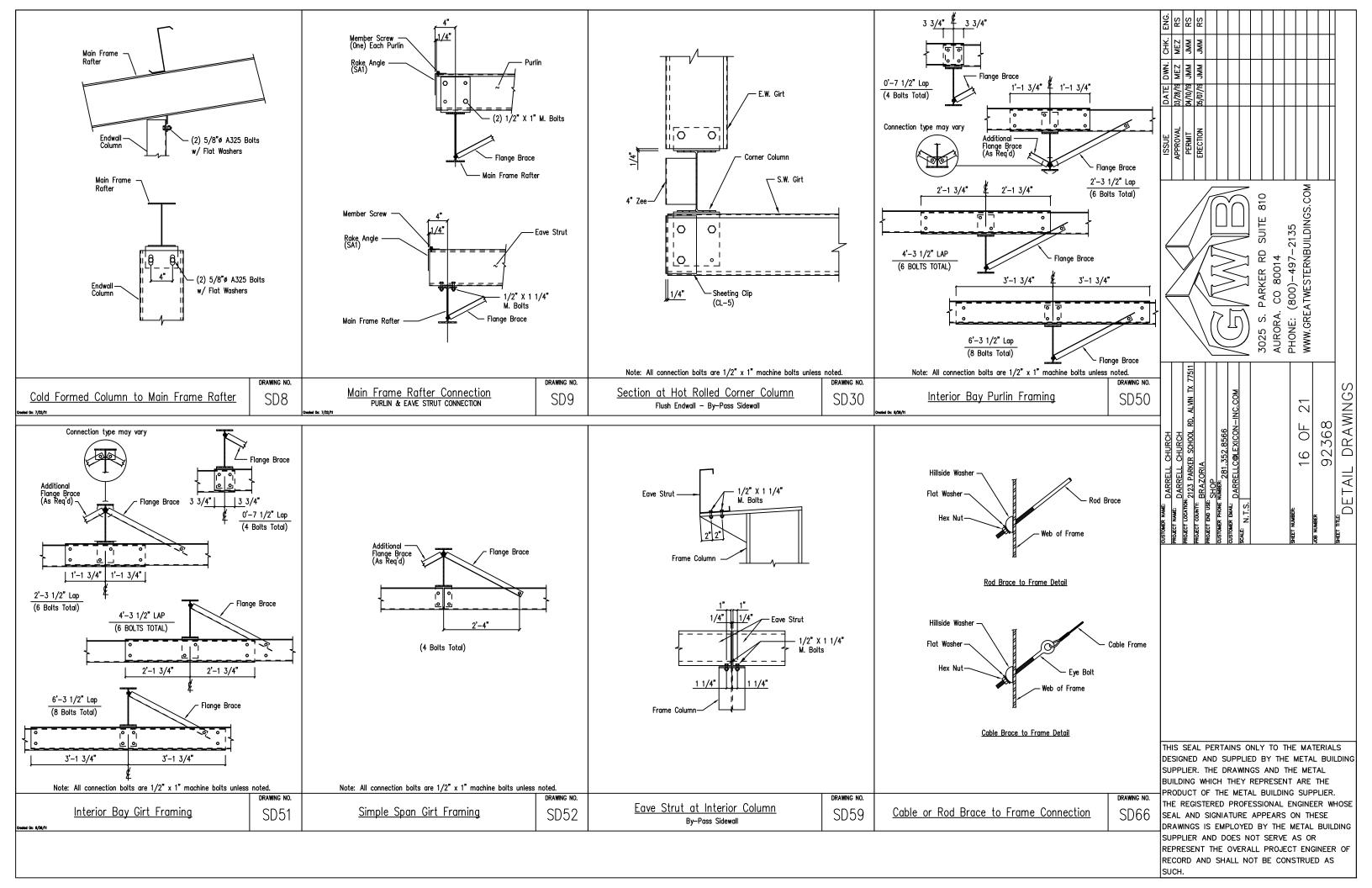


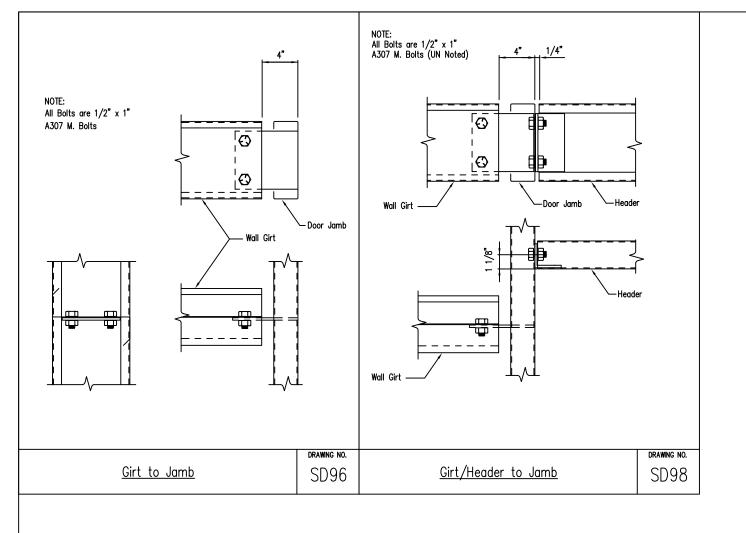




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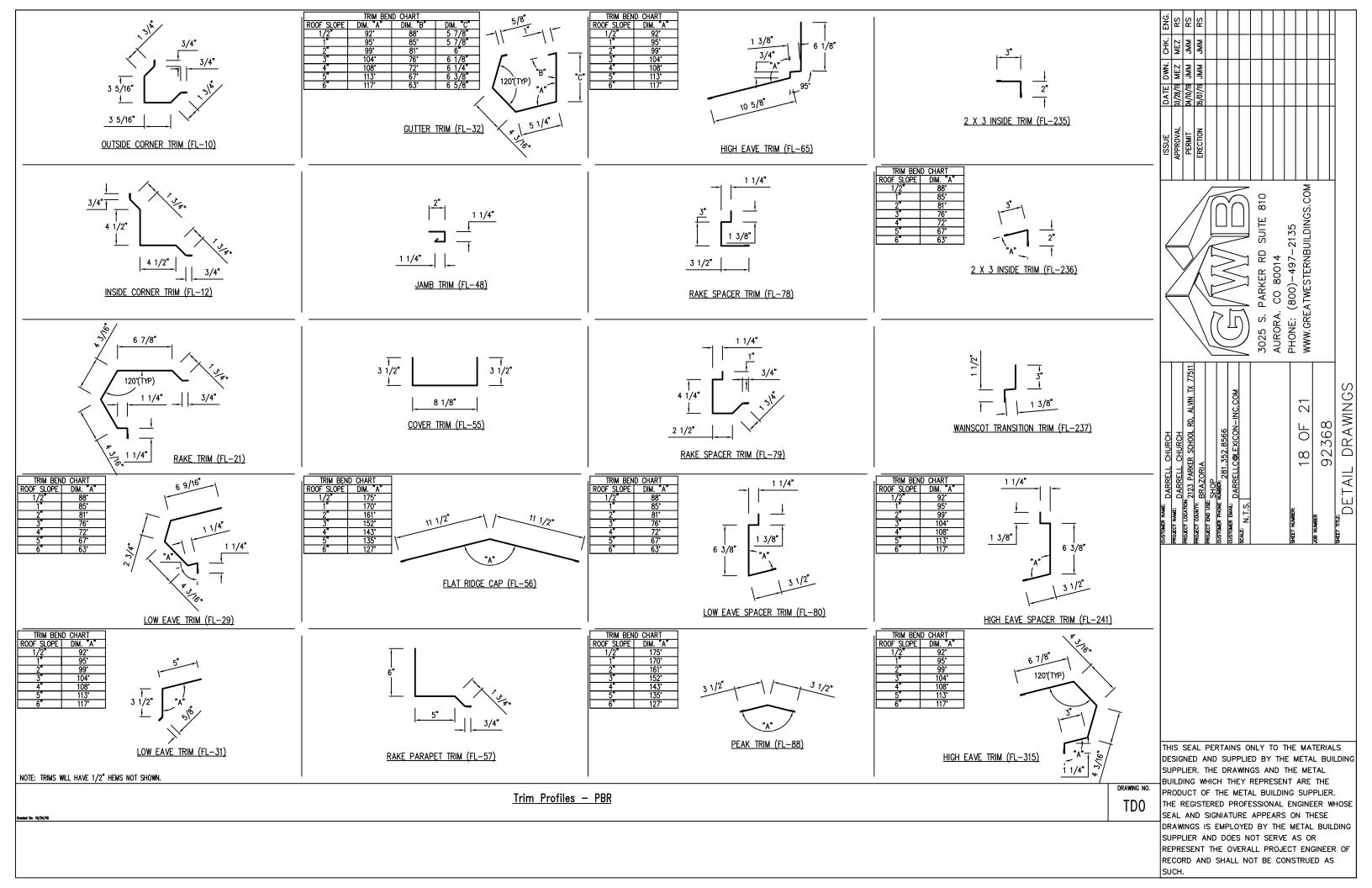


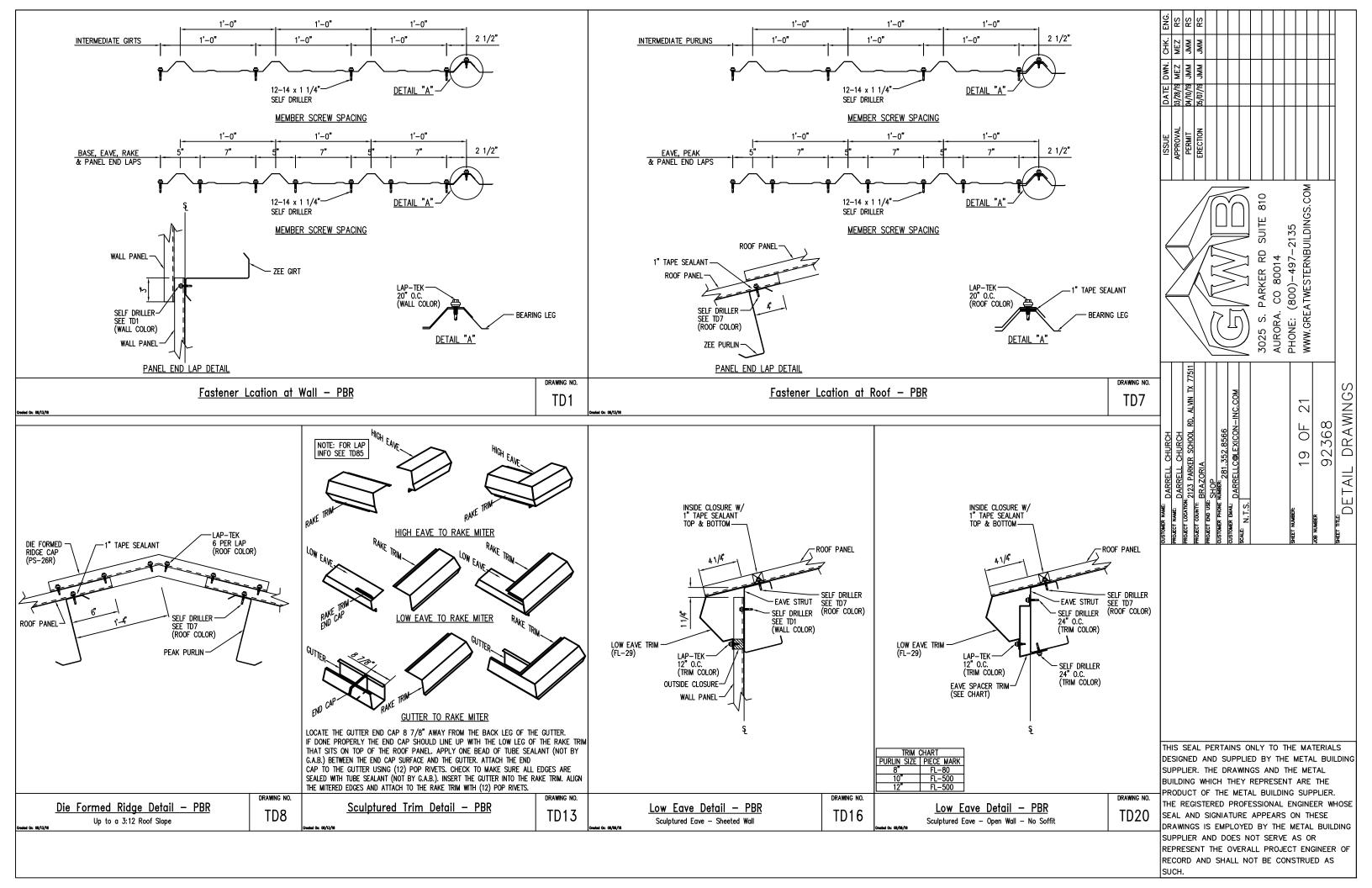


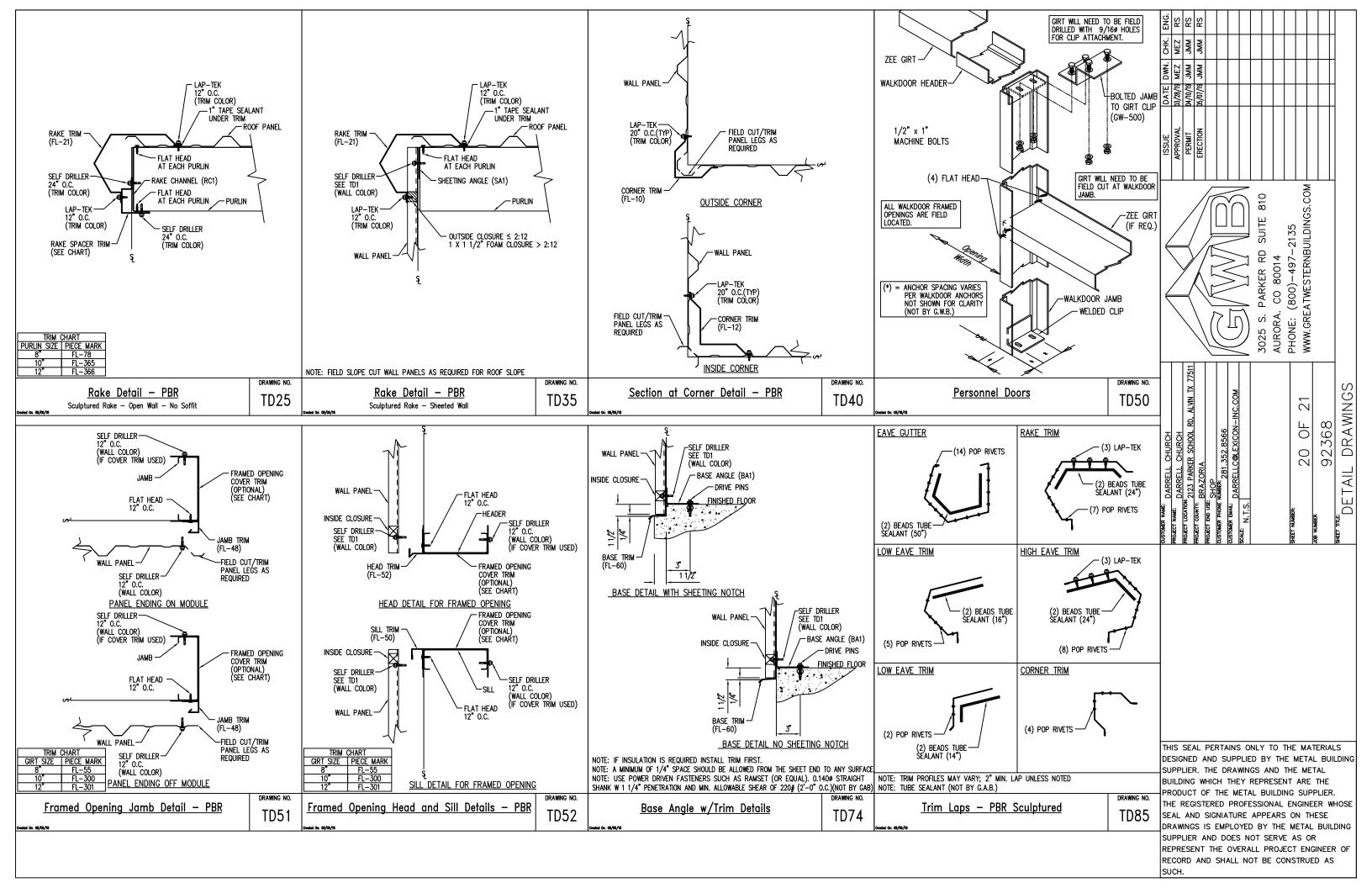


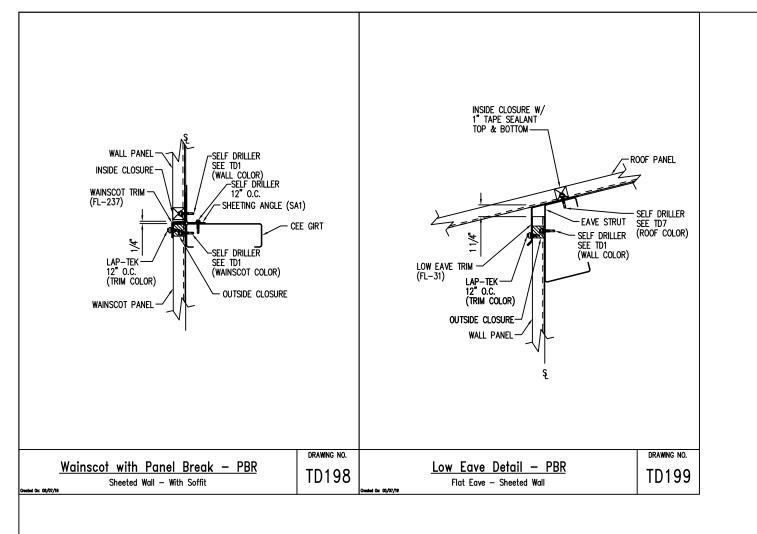
CUSTOMER NAME: DARRELL CHURCH	<b>←</b>	ISSNE	DATE DWN. CHK. ENG.		ENG.
PROJECT NAME: DARRELL CHURCH		APPROVAL	03/28/19 MEZ	MEZ	RS
PROJECT LOCATION: 2123 PARKER SCHOOL RD, ALVIN TX 77511	/ - -	PERMIT	04/10/19 JMM	MMS	RS
PROJECT COUNTS: BRAZORIA		ERECTION	D5/07/19 JMM	MMC	RS
PROJECT END USE: SHOP					
CUSTOMER PHONE NUMBER: 281,352,8566					
CUSTOMER EMAIL: DARRELLC@LEXICON—INC.COM					
SCALE: N. T. S.					
	3036 3 37/040 3 3005				
	JUZJ S. PARNER RD SUITE OIU				
	AURORA. CO 80014				
SHEET NUMBER:	PHONE: (800)-497-2135				
17 OF 21	WWW GREATWESTERNBIII DINGS COM				
JOB NUMBER					
92368					
DETAIL DRAWINGS					

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APPROVAL	. D3/28/19 MEZ	EZ MEZ	Z RS
PERMIT	MMC 81/01/40	MMC MM	M RS
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	3025 S. PARKER RD SUITE 810 AURORA. CO 80014 PHONE: (800)-497-2135 WWW.GREATWESTERNBUILDINGS.COM	10 COM	COM COM

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