

LEGEND EXISTING WALL

WALL TO BE REMOVED SMOKE DETECTOR VENT

CARBON MONOXIDE ALARM

Smoke detectors:

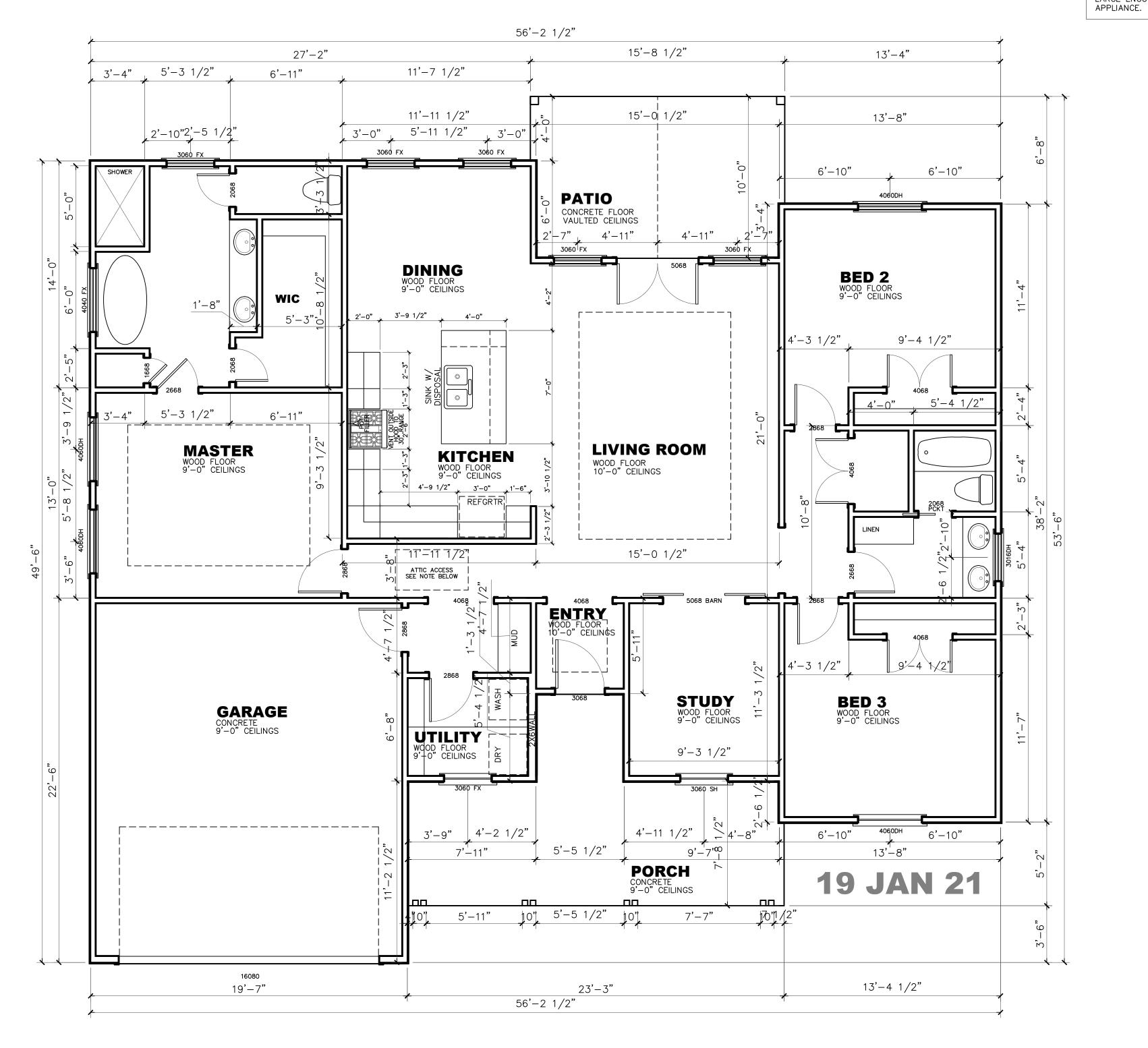
new contruccion hard—wired w/battery backup.(317.2)
Req's in each bedroom & adjoining hall.(317.1)
Must be interconnected and audible from sleepeng rooms.(317.1)

Attic Access:

THE ATTIC ACCESS STAIRWAY SHALL COMPLY WITH SECTION M1305.1.3 AS AMMENDED BY THE CITY OF HOUSTON. THE REQUIREMENTS HAVE BEEN REVISED AS SUCH.

ATTICS CONTAINING APPLIANCES REQUIRING ACCESS SHALL BE PROVIDED WITH A PULL DOWN STAIRWAY WITH A CLEAR OPENING NOT LESS THAN 22-INCHES IN WIDTH BETWEEN THE HARDWARE, AND A LOAD CAPACITY OF NOT LESS THAN 350-POUNDS. AN OPENING AND A CLEAR AND UNOBSTRUCTED PASSAGEWAY LARGE ENOUGH TO ALLOW REMOVAL OF THE LARGEST APPLIANCE, BUT NOT LESS THAN 30-INCHES HIGH AND 30-INCHES WIDE AND NOT MORE THAN 20-FEET IN LENGTH WHEN MEASURED ALONG THE CENTERLINE OF THE PASSAGEWAY FROM THE OPENING TO THE APPLIANCE.

THE PASSAGEWAY SHALL HAVE CONTINUOUS SOLID FLOORING IN ACCORDANCE WITH CHAPTER 5; NOT LESS THAN 24-INCHES WIDE. A LEVEL SERVICE SPACE AT LEAST 30-INCHES WHERE ACCESS IS REQUIRED. THE CLEAR ACCESS OPENING DIMENSIONS SHALL BE A MINIMUM OF 30-INCHES BY 54-INCHES WHERE SUCH DIMENSIONS ARE LARGE ENOUGH TO ALLOW REMOVAL OF THE LARGES







1,814 SQ/FT 435 SQ/FT GARAGE FRONT PORCH REAR PATIO 208 SQ/FT 154 SQ/FT

TOTAL LIVING: TOTAL AREA:

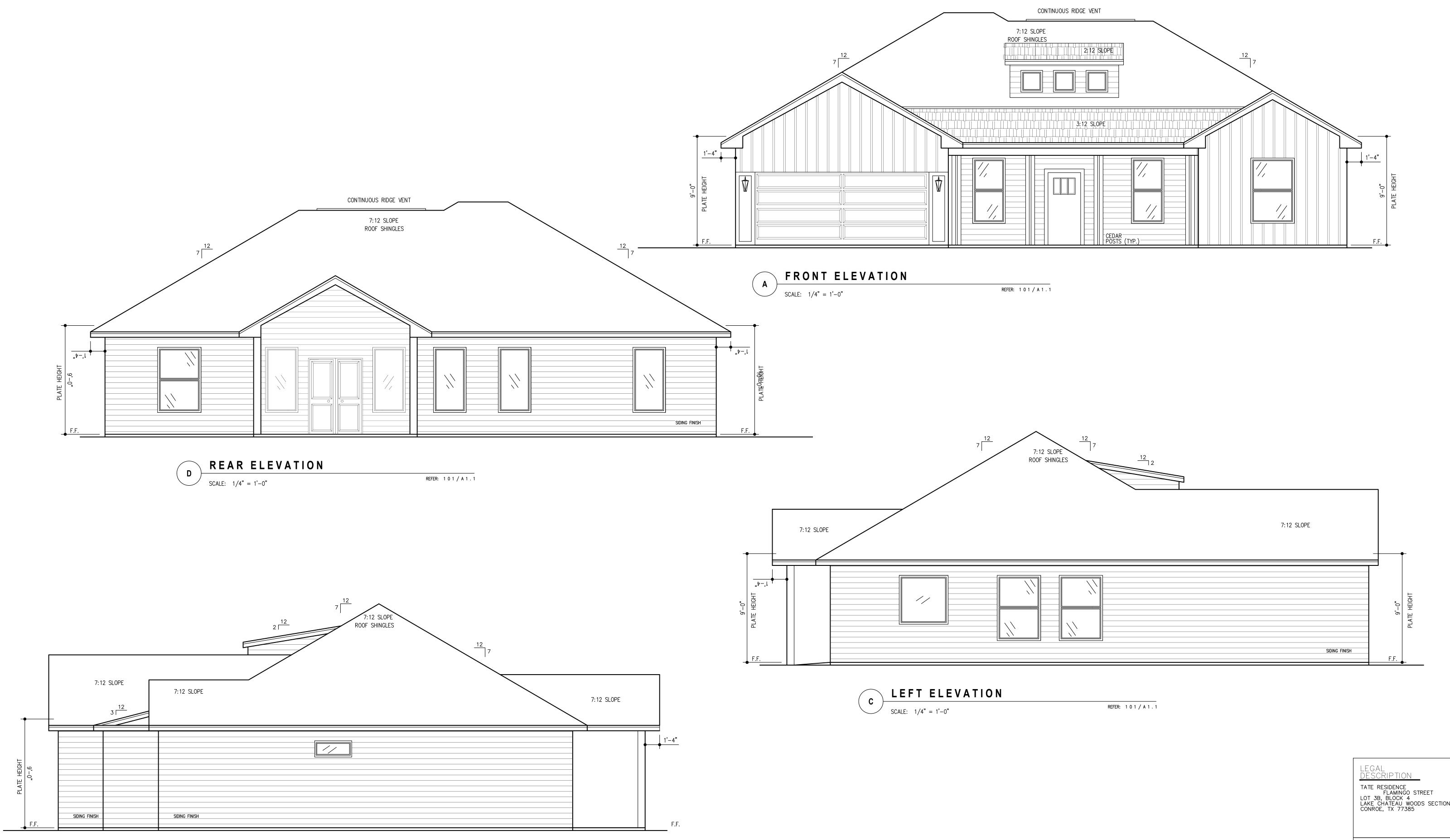
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19 JAN 21 1 OF 5

797 SQ/FT

2,611 SQ/FT





RIGHT ELEVATION

REFER: 101/A1.1

LEGAL
DESCRIPTION

TATE RESIDENCE
FLAMINGO STREET
LOT 3B, BLOCK 4
LAKE CHATEAU WOODS SECTION 7
CONROE, TX 77385

SQUARE FOOTAGES
LIVING
1,814 SQ/FT
GARAGE 435 SQ/FT
FRONT PORCH 208 SQ/FT
REAR PATIO 154 SQ/FT

TOTAL MISC AREA: 797 SQ/FT
TOTAL AREA: 2,611 SQ/FT

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- 1. FOUNDATION SHOWN ON THE ACCOMPANYING DRAWING HAS BEEN DESIGNED USING ACCEPTABLE ENGINEERING PRACTICES AND IS IN ACCORDANCE WITH THE CRITERIA FOR SELECTION AND DESIGN OF RESIDENTIAL SLABS-ON-GRADE (BRAB REPORT). THE AMERICAN CONCRETE INSTITUTE BUILDING CODE REQUIREMENTS FOR REINFOR-CED CONCRETE AND/OR THE DESIGN AND CONSTRUCTION OF POST- TENSIONED SLABS-ON-GROUND BY THE POST-TENSIONED INSTITUTE.
- 2. THE GENERAL CONTRACTOR HAS, BY USE OF THESE PLANS AND GENERAL NOTES, ACCEPTED THE RECOMMENDATIONS AND METHODS WE HAVE RELIED ON FOR OUR DESIGN AS PART OF HIS CONSTRUCTION PROCEDURE.

3. N/A

COORDINATION

- 4. THE GENERAL CONTRACTOR MUST VERIFY ALL DROPS, OFFSETS, BRICK LEDGES, AND BLOCKOUTS ON THE ARCHITECTURAL PLANS AND NOTIFY THE ENGINEER OF ANY DISCREPANCIES THAT MAY EXIST ON STRUCTURAL PLANS. ENGINEER SHALL NOT BE HELD LIABLE FOR ANY DIMENSIONAL ERRORS ONCE CONSTRUCTION HAS
- 5. THE GENERAL CONTRACTOR MUST COORDINATE THE STRUCTURAL PLANS EITH ARCHITECTURAL AND MECHANICAL DRAWINGS FOR ALL OPENINGS, INSERTS AND OTHER RELATED ITEMS REQUIRED TO COM-PLETE THE FOUNDATION.

MATERIALS

- 6. CONCRETE IN FOUNDATION BEAMS AND SLABS TO ATTAIN A MINI-MUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS AND AT LEAST 2000 PSI AT TIME OF STRESSING (3 TO 10 DAYS). CONCRE-TE DESIGN MIX SHALL BE IN ACCORDANCE WITH THE A.C.I. BUILDING CODE REQUIREMENTS (ACI 318-83) TO INSURE QUALITY CONCRETE. USE MINIMUM 4 1/2 SACKS OF CEMENT PER CUBIC YARD WITH 5 1/2" MAXIMUM SLUMP. SUBMIT COMPRESSIVE TESTS TO ENGINEER. CURING IS REQUIRED UNLESS NOTED OTHERWISE ON PLANS. CALCIUM CHLORIDE OR ADMIXTURES CONTAINING CALCIUM CHLORIDE SHALL NOT BE USED AS ADDITIVES IN CONCRETE MIX.
- 7. PLACE A 6 MIL PLYTHYLENE VAPOR BARRIER UNDER ALL CONCRETE SLABS.
- 8. PRESTRESSING TENDONS SHALL CONSIST OF SEVEN WIRE STRESS RE-LIEVED CABLE CONFORMING TO ASTM A-416, MINIMUM ULTIMATE TEN-SITE STRENGTH, PREVENTIVE LUBRICANT AND WRAPPED WITH PLASTIC SHEATHING. ALL END ANCHORAGE DEVICES SHALL CONFORM TO P.T.I. DESIGN SPECIFICATIONS. ALL DEAD END ANCHORAGES SHALL BE FAC-TORY SEATED OR POWER SEATED.
- 9. MILD STEEL SHALL CONFORM TO ASTM A-615, GRADE 60 FOR #4 AND LARGER, GRADE 40 FOR #3 OR #2. WIRE FABRIC SHALL CONFORM TO
- 10. PROVIDE 4" MINIMUM CUSHION LAYER UNDER THE SLAB OF EITHER PERVIOUS SAND OR GRANULAR FILL, PER THE SOIL REPORT.

1/2"ø x 8" LG. @ 48" —ANCHOR BOLT

N.G.

SECTION F-F

SECTION G-G

12" + 6"

TYP. TENDON CHAIR-

N.G.

TENDON SPACING-PER PLAN

CONSTRUCTION

11. BEAM AND SLAB DIMENSIONS ARE THE MINIMUM SIZE REQUIRED AND MAY NOT BE REDUCED OR ENLARGED WITHOUT PRIOR APPROVAL BY THE EN-GINEER.

- 12. PLACE FILL UNDER SLABS IN COMPLIANCE WITH F.H.A. DATA SHEET 796 AND/OR SOIL ENGINEER'S SPECIFICATIONS FOR THE FILL. SOIL ENGINEER WILL CERTIFY COMPLIANCE ON REQUEST OF THE STRUCTURAL ENGINEER.
- 13. CONCRETE SHALL BE WELL CONSOLIDATED, ESPECIALLY AT VICINITY OF
- TENDON ANCHORAGE LOCATIONS. 14. SUPPORT TENDONS AT 4'-6" CENTERS MAXIMUM IN BOTH DIRECTIONS TO PREVENT VERTICAL AND HORIZONTAL DISPLACEMENT DURING PLACING OF CONCRETE. ALLOWABLE TOLERANCES: +/-1/2" VERTICALLY; +/-6"
- 15. ALLOW STRESSING EQUIPMENT CLEARANCE OF 8" DIAMETER ON TENDON
- AXIS BY 36" LENGTH. 16. IF TENDON SHEATING IS DAMAGED FOR 3" OR MORE, IT MUST BE RESHEA-
- THED TO PREVENT CONCRETE FROM BONDING TO THE STRAND. 17. WE CANNOT BE HELP RESPONSIBLE FOR THE ADEQUANCY OF CONSTRUC-TION OR COMPLIANCE TO THESE DRAWINGS UNLESS WE ARE CONTRACTED TO PROVIDE REGULAR INSPECTIONS DURING CONSTRUCTION AND THERE BY HAVE LIMITED CONTROL OVER FIELD APPLICATION.
- QUALIFICATIONS 18. POST-TENSION CONTRACTOR SHALL FURNISH THE FOLLOWING TO THE CON-TRACTOR AND THE STRUCTURAL ENGINEER:
- A: LABORATORY TEST ON ANCHORAGE SYSTEM
- B: LATEST CALIBRATION DATE OF EQUIPMENT. C: LABORATORY TEST COEFFICIENT OF FRICTION ON STRAND.
- D: MIL TEST CERTIFICATE ON STRAND. E: TENDON ELONGATION LOGS FOR EACH BUILDING.

STRESSING

19. USE 1/2" DIAMETER SEVEN STRAND TENDONS RATED AT 270 KSI TENSILE STRESSED AT 28.9K PER STRAND, BUT MAY BE INITIALLY STRESSED AT 33K PER STRAND ELONGATION TO BE .079 INCHES PER FOOT (MINIMUM) OF TENDON LENGHT.

SPECIAL NOTES

20. EXTERIOR BEAMS ARE TO EXTEND A MINIMUM OF 9" INTO UNDISTURBED.

22. SLOPE THE GRADE AND DRAIN WATER AWAY FROM ALL BUILDING FOUNDA-TIONS. MINIMUM 6" SLOPE IN 10' DISTANCE UNLESS NOTED OTHERWISE.

ANCHOR BOLTS

1/2"ø x 8" LG. @ 48" —ANCHOR BOLT

_4" SAND 2'−0"

TYP. TENDON CHAIR

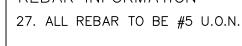
_TENDON SPACING PER PLAN

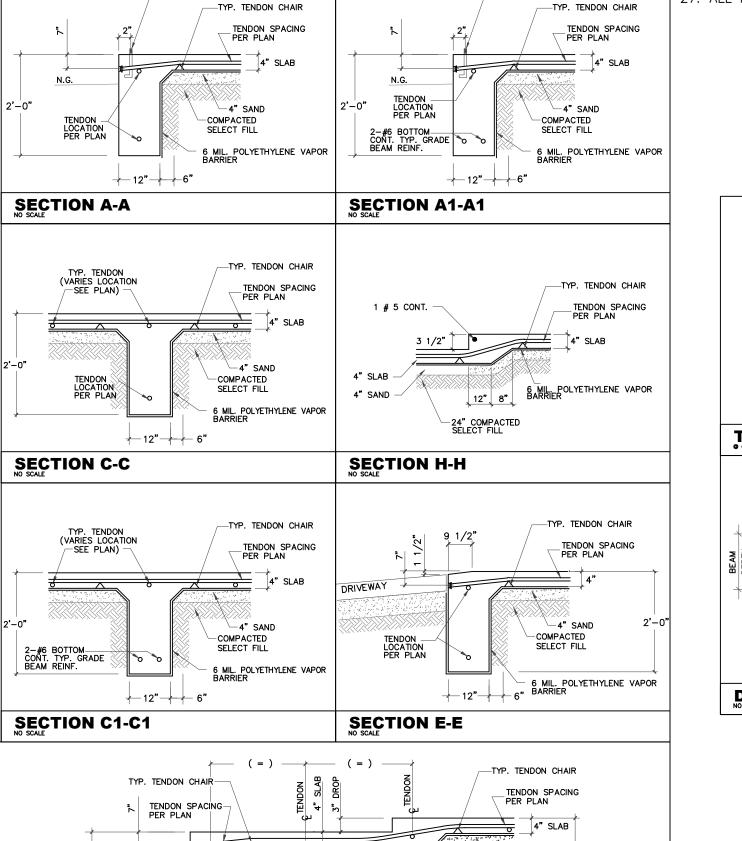
4" SAND 2'-0"

4" SLAB

- 23. EXTERIOR WALLS: USE 1/2" DIAMETER BY 10" J-BOITS AT 48" CENTERS MAXIMUM WITH 7" MINIMUM EMBEDMENT INTO CONCRETE UNLESS NOTED
- 24. INTERIOR BEARING WALLS: USE HILTI POWER-DRIVEN FASTENERS 145" DIA-METER X 1 1/2" PENETRATION AT 18" MAXIMUM CENTERS OR 1/2" DIAME-TER X 10" J-BOLTS AT 48" CENTERS MAXIMUM UNLESS NOTED OTHERWISE
- ON PLANS. 25. INTERIOR NON-BEARING WALLS: USE HILTI POWER-DRIVEN FASTENERS 145" DIAMETER X 1 1/2" PENETRATION AT 24" CENTERS UNLESS NOTED OTHERWISE.
- GRADE BEAM INFORMATION
- 26. ALL EXTERIOR GRADE BEAMS TO BE 12" (16" WHERE LEDGE). ALL INTERIOR GRADE BEAMS TO BE 12"

REBAR INFORMATION



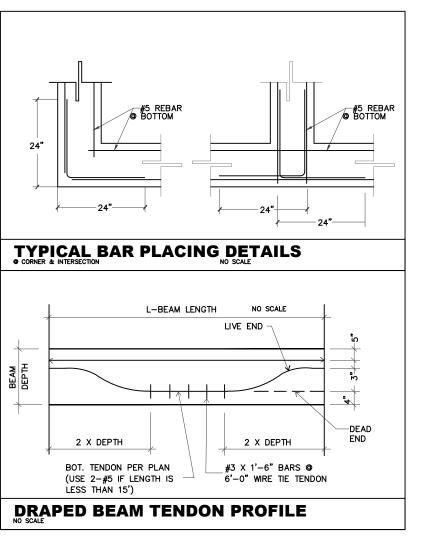


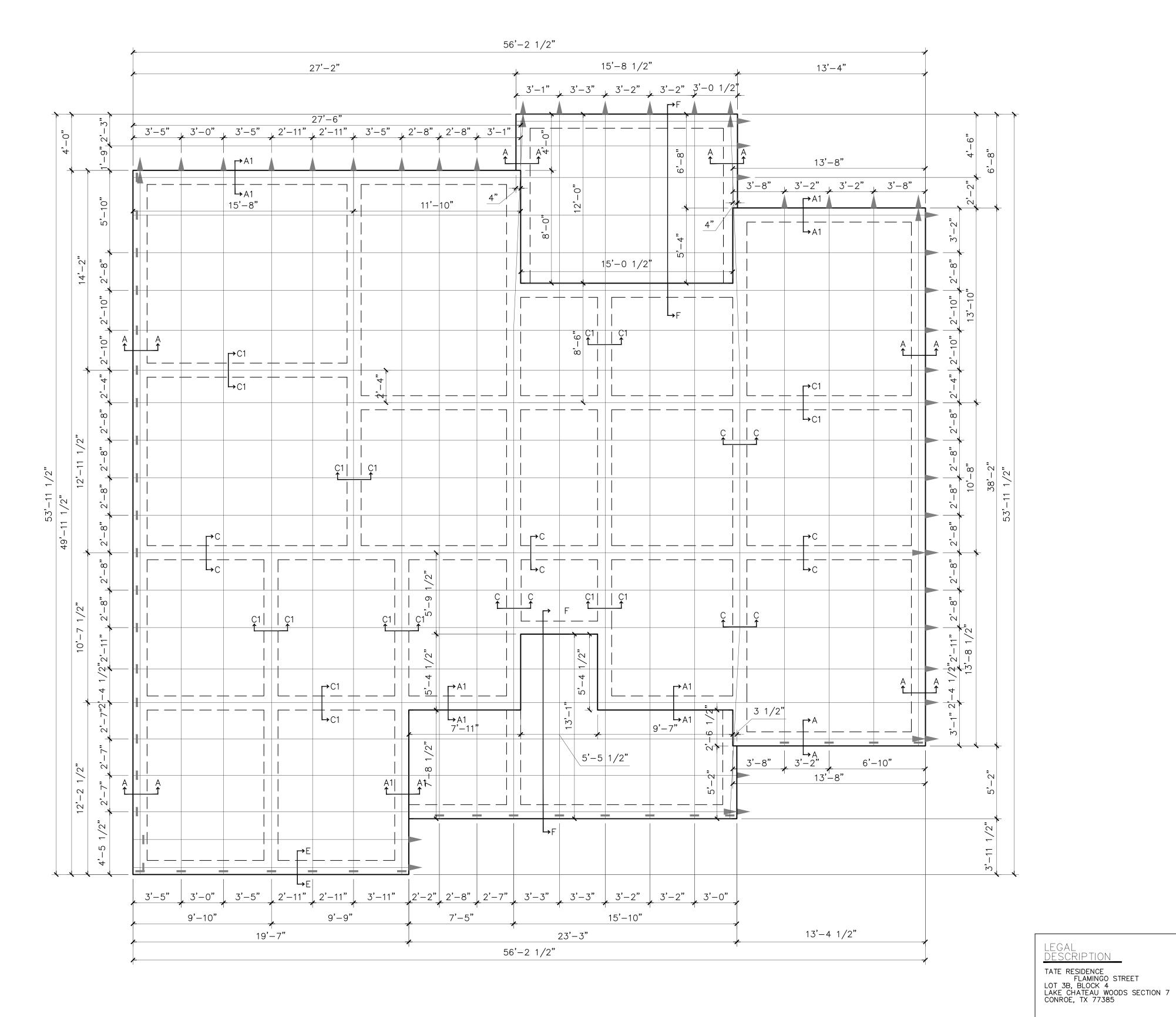
└─4" SAND

∤ (=) → (=) →

^{_}4" SAND

6 MIL. POLYETHYLENE – 12" 6" VAPOR BARRIER





FOUNDATION

SCALE: 1/4"=1'-0"

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TOTAL AREA:

GARAGE

FRONT PORCH

REAR PATIO

TOTAL LIVING:

2,611 SQ/FT 19 JAN 21 **S1**

SQUARE FOOTAGES

1,814 SQ/FT

435 SQ/FT

208 SQ/FT

154 SQ/FT

797 SQ/FT

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NAILING SCHED)ULE
TABLE NO.25-Q-NAILING SCHEDULE (2	2012 IRC)
DESCRIPTION	NAILING (1)
1) JOIST TO SILL OR GIRDER, TOENAIL	3-8d
2) BRIDGING TO JOIST, TOENAIL EACH END	2-8d
3) 1" X 6" SUBFLOOR OR LESS TO EACH JOIST, FACE NAIL	2-8d
4) WIDER THAN 1" X 6" SUBFLOOR OR LESS TO EACH JOIST,	FACE NAIL 3-8d
5) 2" SUBFLOOR TO JOIST OR GIRDER, BLIND AND FACE NAIL	2-16d
6) SOLE PLATE TO JOIST OR BLOCKING, FACE NAIL	16d AT 16" O.C.
7) TOP PLATE TO STUD, END NAIL	2-16d
	4-8, TOENAIL OR 2-16d, END NAIL
9) DOUBLE STUDS, FACE NAIL 10) DOUBLED TOP PLATES, FACE NAIL	16d at 24" o.c.
11) TOP PLATES, LAPS AND INTERSECTIONS, FACE NAIL	16d at 16" o.c. 2—16d
· · · · · · · · · · · · · · · · · · ·	AT 16" O.C. ALONG EACH EDGE
13) CEILING JOISTS TO PLATE, TOENAIL	3-8d
14) CONTINUOUS HEADER TO STUD, TOENAIL	4-8d
15) CEILING JOISTS, LAPS OVER PARTITIONS, FACE NAIL	3–16d
16) CEILING JOISTS TO PARALLEL RAFTERS, FACE NAIL	3–16d
17) RAFTER TO PLATE, TOENAIL	3-8d
18) 1" BRACE TO EACH STUD AND PLATE, FACE NAIL	2-8d
19) 1" X8" SHEATHING OF LESS TO EACH BEARING, FACE NAIL	2-8d
20) WIDER THAN 1"X 8" SHEATHING TO EACH BEARING, FACE N	NAIL 3-8d
21) BUILT-UP CORNER STUDS	16d at 24" o.c.
	.T 32" O.C. AT TOP & BOTTOM AND ED 2—20d AT ENDS AND @ EACH SPLICI
23) 2" PLANKS	2-16d AT EACH BEARING
24) PLYWOOD AND PARTILCEBOARD: (5) SUBFLOOR, ROOF, AND WALL SHEATHING (TO FRAMING) 1/2" AND LESS	6d (2)
19/32" -3/4"	8d (3)or 6d (4)
7/8–1"	8d (2)
1 1/8–1 1/4"	10d(3)or 8d (4)
COMBINATION SUBFLOOR-UNDERLAYMENT (TO FRAMING)	.04(0/0. 04 (1)
3/4" AND LESS	6d (4)
7/8–1"	8d (4)
1 1/8-1 1/4"	10d(3)or 8d (4)
25) PANEL SIDING (TO FRAMING): (7) 1/2" OR LESS	6d (6)
5/8"	8d (6)
26) FIBERBOARD SHEATHING	No. 11 ga(8)
1/2"	6d (3) No. 16 ga.(9)
25/32"	No. 11 ga(8)
	8d (3) No. 16 ga.(9)
NAILING SCHEDULE NOTES: (1) COMMON OR BOXED NAILS MAY BE USED EXCEPT WHERE OT (2) COMMON OR DEFORMED SHANK. (3) COMMON. (4) DEFORMED SHANK. (5) NAILS PLACED AT 6 INCHES ON CENTER AT EDGES, 12 INCHEXCEPT 6 INCHES AT ALL SUPPORTS WHERE SPANS ARE 48 OF PLYWOOD AND PARTICLEBOARD DIAPHRAGMS AND SHEAR 2513 (c). NAILS FOR WALL SHEATHING MAY BE COMMON, BOX OF CORROSION—RESISTANT SIDING OR CASING NAILS CONFORMIN	HES AT INTERMEDIATE SUPPORTS 3 INCHES OR MORE. FOR NAILING R WALLS, REFER TO SECTION OX, OR CASING.
OF SECTION 2516 (j) 1. (7) FASTENERS SPACED 3 INCHES ON CENTER AT EXTERIOR EDG	
AT INTERMEDIATE SUPPORTS. (8) CORROSION—RESISTANT ROOFING NAILS WITH 7/16 INCH—DIA LENGTH FOR 1/2 INCH SHEATHING AND 1 3/4 INCH LENGTH CONFORMING TO THE REQUIREMENTS OF SECTION 2516 (j) 1	H FOR 25/32 INCH SHEATHING

- 1. HIP, VALLEY, AND RIDGE SHALL ALWAYS BE ONE SIZE LARGER THAN RAFTERS.
- 2. PROVIDE COLLAR TIES AT UPPER 1/3 DISTANCE BETWEEN RIDGE BOARD AND RIDGE AT 48" O.C.
- 3. ALL RAFTERS 2 X 6 @ 16" O.C. (U.N.O.) #2 SYP 4. DOUBLE FLOOR JOIST UNDER ALL PARTITIONS PARALLEL TO JOIST BELOW.
- 5. PROVIDE CROSSBRIDGING @ 1'-0" O.C. ON ALL 2X12 JOISTS.
- 6. PROVIDE RAFTER TIES AT ALL PLATE WHERE JOISTS ARE PERPENDICULAR TO RAFTER.

7. PROVIDE 2-2X STRONGBACKS ON SPANS OVER 10'-0". 8.ROOF FRAMING:

17.ALL FRAMING LUMBER TO BE #3 SYP (U.O.N.)

MAXIMUM UNSUPORTED SPAN FOR RAFTERS SHALL BE 10'-0". ALL ROOF BRACING SHALL BE SUPPORTED BY A WALL, 2-2X6 STRONGBACK, OR 2-2X12 DEPENDING ON CEILING JOIST DIRECTION (PROVIDE BLOCKING AT BRACE LOCATION), U.O.N., MAXIMUM ANGLE FOR 2X4 BRACES IN ATTIC SHALL BE 45 FROM VERTICAL. WHERE LENGTH OF BRACING EXCEEDS 8'-0", PROVIDE ALTERNATE BRACING METHODS AS DESCRIBED ON ATTACHED SHEET. 9.PROVIDE LEAD COATED COPPER FLASHING AT ALL VALLEYS, HIPS, & RIDGES WHERE APPLICABLE. ALSO APPLY FOR PROJECTING THROUGH ROOF WITH FLANGE AND EXTEND FLANGE 8" BEYOND SLEEVE. 10.ALL BEAM AND HEADER MATERIAL SHALL BE #2 SYP. ALL RAFTER AND JOIST MATERIAL SHALL BE #2 SYP. 11.ALL WALL STUDS SHALL BE STUD GRADE FIIR 16" O.C. 12.ALL STEEL SHALL CONFORM TO ASTM A-36.

13.LIVE LOADS:

ROOF - 16 PSF
SECOND FLOOR - 40 PSF

ATTIC STORAGE - 30 PSF

14.SUPPORT ALL JOIST ON BEAMS WITH SIMPSON U-JOIST METAL HANGERS, UNLESS OTHERWISE NOTED,

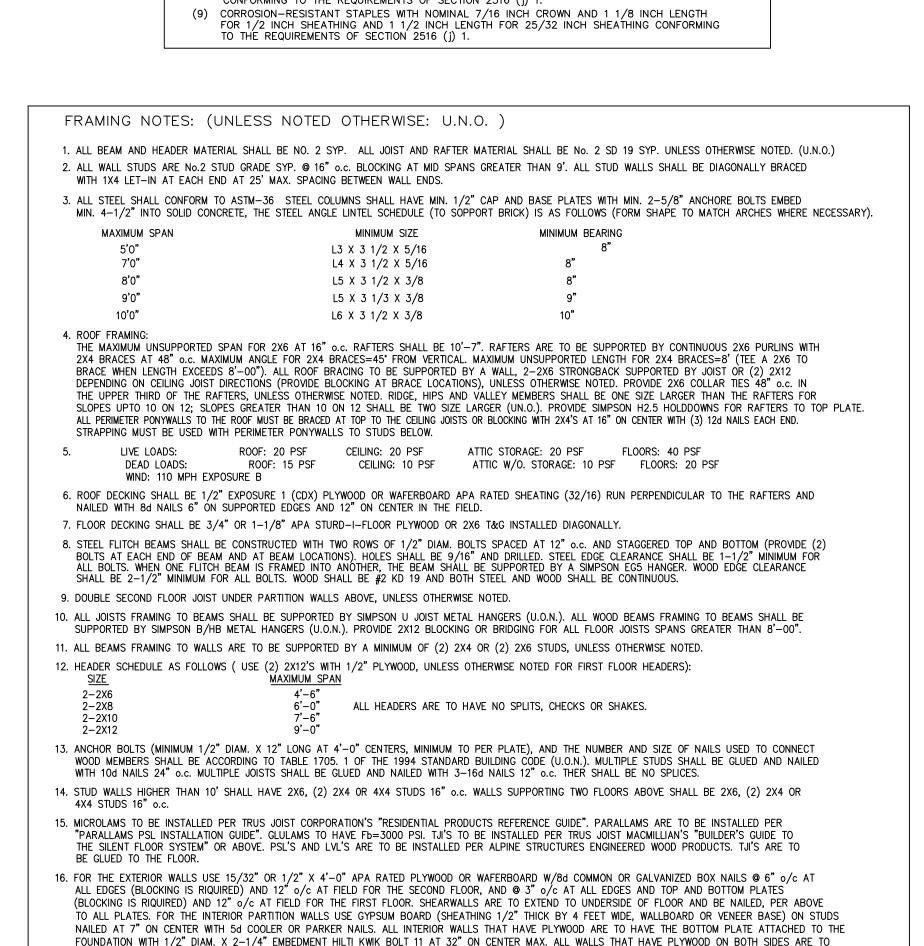
SUPPORT ALL BEAMS ON OTHER BEAMS WITH SIMPSON B/HB METAL HANGERS (U.N.O.)

15.ALL BEAMS FRAMING TO WALL ARE TO BE SUPPORTED BY A MINIMUM OF 2-2X4 OR 2-2X6 STUDS (U.O.N.).

16.ALL FRAMING DESIGNED TO SUSTAIN 110MPH 3SECOND WIND GUSTS.

FRAMING NOTES

SCALE: 1/4"=1'-0"



HAVE A SIMPSON HD8A TO DOUBLE STUDS AT THE ENDS OF THE PLYWOOD AND BE DOUBLE BLOCKED AND DOUBLE STUDDED.

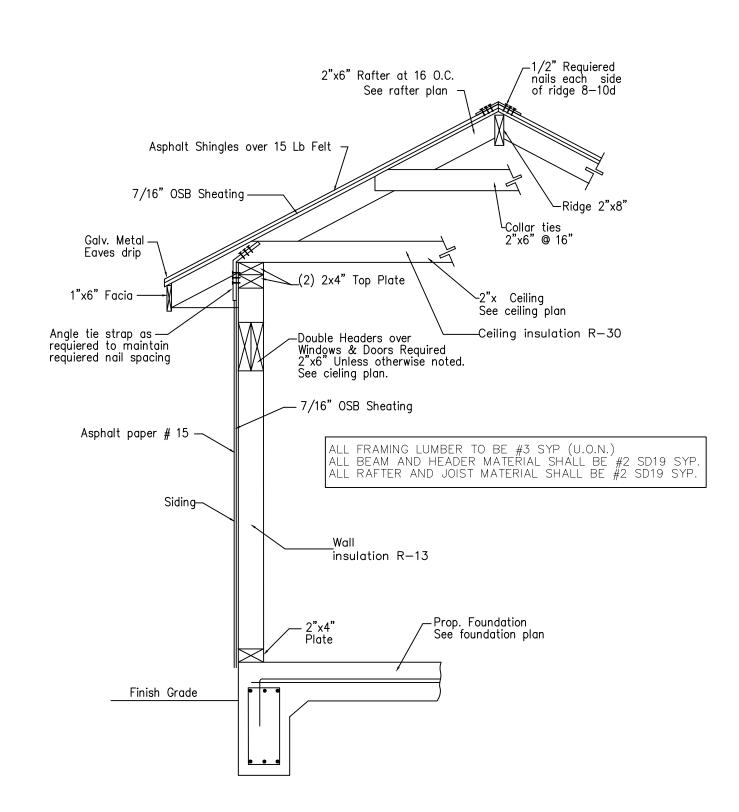
NAILS 12"o.c. THERE SHALL BE NO SPLICES.

17. THE NUMBER AND SIZE OF NAILS USED TO CONNECT WOOD MEMBERS, SHALL BE ACCORDING TO TABLE 25Q OF THE HOUSTON/UBC BUILDING CODE IS

18. CONTRACTOR/OWNER SHALL VERIFY FIELD DIMENSIONS AND DETAILS , NOTIFY THE PROJECT ARCHITECT/ENGINEER OF ANY DISCREPANCY AND REVIEW FOR

RECOMMENDÁTION OR REVISIONS IF NECESSARY. ALL CONSTRUCTION PROCEDURES SHALL CONFORM TO LOCAL CODES AND OSHA GUIDELINES.

APPLICABLE (U.N.O. MULTIPLE STUDS SHAL BE GLUED AND NAILED WITH 10d NAILS 24" o.c. MULTIPLE JOIST SHALL BE GLUED AND NAILED WITH 3-16d



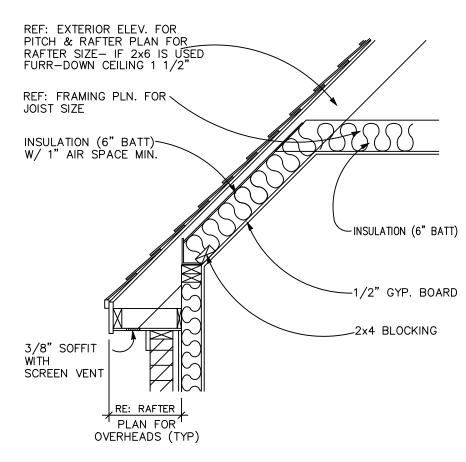
WALL SECTION
SCALE: 1/4"=1'-0"



CEILING FRAMING PLAN

SCALE: 1/4"=1'-0"

1. PLATE HEIGHT AT 9'-0".
2. ALL FRAMING LUMBER TO BE #3 SYP (U.O.N.)
3. JOISTS SHALL BE SUPPORTED LATERALLY AT EACH END AND AT EACH SUPPORT.
4. SOLID BLOCKING SHALL NOT BE LESS THAN 2" IN THICKNESS AND MUST BE THE FULL DEPTH OF JOIST.



RAISED CEILING

LEGAL DESCRIPTION TATE RESIDENCE FLAMINGO STREET LOT 3B, BLOCK 4 LAKE CHATEAU WOODS SECTION 7 CONROE, TX 77385 SQUARE FOOTAGES 1,814 SQ/FT GARAGE 435 SQ/FT FRONT PORCH 208 SQ/FT REAR PATIO 154 SQ/FT 797 SQ/FT TOTAL LIVING: TOTAL AREA: 2,611 SQ/FT 19 JAN 21 **TEXAS PERMITS** PLANS & PERMITS PH: (713) 245-0779 FX: (832) 201-0818

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APPENDIX L

CONVENTIONAL LIGHT-FRAME WOOD CONSTRUCTION FOR SINGLE FAMILY RESIDENTIAL CONSTRUCTION IN HIGH-WIND AREAS SECTION AL101

GENERAL

AL101.1 Scope. This chapter applies to regular—shaped single family residential buildings that are not more than three stories in height and are of conventional light-frame construction. Exception: Detached carports and garages not exceeding 700 square feet (65 m²) and accessory to Group R-3 occupancies need only comply with the roof-member- to-wall- tie requirements of Section AL103.8.

SECTION AL102

DEFINITION

CORROSION RESISTANT or NONCORROSIVE. Refers to a material having a corrosion resistance equal to or greater than a hot-dipped galvanized coating of 1.5 ounces of zinc per square foot (4 g/m^2) of surface area. When an element is required to be corrosion resistant or noncorrosive, all of its parts, such as screws, nails, wire, dowels, bolts, nuts, washers, shims, anchors, ties and attachments, shall also be corrosion resistant or noncorrosive. SECTION AL103

COMPLETE LOAD PATH AND UPLIFT TIES

AL103.1 General. Blocking, bridging, straps, approved framing anchors or mechanical fasteners shall be installed to provide continuous ties from the roof to the foundation system. Tie straps shall be 11/6-inch (28.6 mm) by 0.036-inch (0.91 mm) (No. 20 gage) sheet steel and shall be corrosion resistant as herein specified. All metal connectors and fasteners used in exposed locations or in areas otherwise subject to corrosion shall be of corrosion-resistant or noncorrosive material. The number of common nails specified is the total required and shall be equally divided on each side of the connection. Nails shall be spaced to avoid splitting of the

Exception: Pre-manufactured connectors that provide equal or greater tie-down capacity may be used, provided that they are installed in compliance with all the manufacturer's specifications.

AL103.2 Wall—to— foundation tie. Exterior walls shall be tied to a continuous foundation system

or an elevated foundation system in accordance with Section AL105.

AL103.3 Sills and foundation tie. Foundation plates resting on concrete or masonry foundations shall be bolted to the foundation with not less than ½-inch- diameter (13 mm) anchor bolts with 7—inch— minimum (178 mm) embedment into the foundation and spaced not more than 4 feet (1219 mm) on center.

AL103.4 Floor—to— foundation tie. The lowest—level exterior wall studs shall be connected to the foundation sill plate or an approved elevated foundation system with bent tie straps spaced not more than 32 inches (813 mm) on center. Tie straps shall be nailed with a minimum of 4 ten penny nails.

AL103.5 Wall framing details. The spacing of studs in exterior walls shall be in accordance with Chapter 23. Mechanical fasteners complying with this chapter shall be installed at a maximum of 32 inches (813 mm) on center as required to connect study to the sole plates, foundation sill plate and top plates of the wall. The fasteners shall be nailed with a minimum of 8 eight penny nails.

Where openings exceed 4 feet (1219 mm) in width, the required tie straps shall be at each edge of the opening and connected to a doubled full—height wall stud. When openings exceed 12 feet (3658 mm) in width, two ties at each connection or a manufactured fastener designed to prevent uplift shall be provided.

AL103.6 Wall sheathing. All exterior walls and required interior main cross-stud partitions shall be sheathed in accordance with Chapter 23.

AL103.7 Floor—to— floor tie. Upper—level exterior wall studs shall be aligned and connected to the wall studs below with tie straps placed a minimum of 32 inches (813 mm) on center and connected with a minimum of 6 eight penny nails per strap.

AL103.8 Roof—members— to—wall tie. Tie straps shall be provided from the side of the roof framing member to the supporting member below the roof. Tie straps shall be placed at every roof-framing member and connected with a minimum of 8 eight penny nails.

AL103.9 Ridge ties. Opposing common rafters shall be aligned at the ridge and be connected at the rafters with tie straps spaced a maximum of 32 inches (813 mm) on center and connected with 8 eight penny nails.

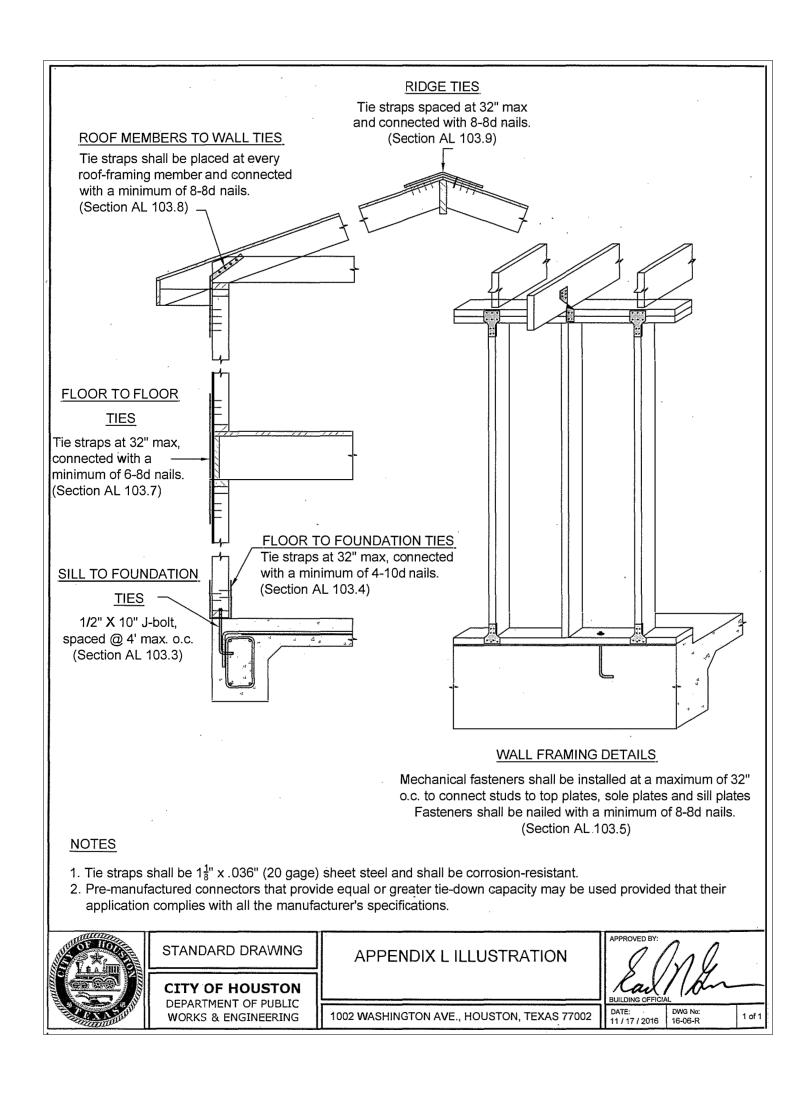
AL103.10 Gable-end walls. Gable-end wall studs shall be continuous between points of lateral support that are perpendicular to the plane of the wall. Gable—end wall studs shall be attached with approved mechanical fasteners at the top and bottom. Eight 8 penny nails shall be required for each fastener. Fasteners shall be spaced a maximum of 32 inches (813 mm) on center. SECTION AL104

ROOFS

AL104.1 Roof sheathing. Solid roof sheathing shall be applied and shall consist of a minimum 1—inch— thick (25.4 mm) nominal lumber applied diagonally or a minimum 15/32—inch— thick (11.9 mm) wood structural panel or particle board (OSB) or other approved sheathing applied with the long dimension perpendicular to supporting rafters. Sheathing shall be nailed to roof framing in an approved manner. The end joints of wood structural panels or particle board shall be staggered and shall occur over blocking, rafters, or other supports.

AL104.2 Roof covering. Roof coverings shall be approved and shall be installed and fastened in accordance with Chapter 15 and with the manufacturer's instructions.

AL104.3 Roof overhang. The roof eave overhang shall not exceed 3 feet (914 mm) unless an analysis is provided showing that the required resistance is provided to prevent uplift. The roof overhang at gabled ends shall not exceed 2 feet (610 mm) unless an analysis showing that the required resistance to prevent uplift is provided.



ELEVATED FOUNDATION

SECTION AL105

AL105.1 General. When approved, elevated foundations supporting not more than one story and meeting the provisions of this section may be used. A foundation investigation may be required by the building official.

AL105.2 Material. All exposed wood-framing members shall be treated wood. All metal connectors and fasteners used in exposed locations shall be corrosion-resistant or noncorrosive

AL105.3 Wood piles. The spacing of wood piles shall not exceed 8 feet (2438 mm) on center. Square piles shall not be less than 10 inches (254 mm) and tapered piles shall have a tip of not less than 8 inches (203 mm). Eight-inch- square (5161 mm²) piles shall have a minimum embedment length of 5 feet (1524 mm) and shall project not more than 8 feet (2438 mm) above undisturbed ground surface. Eight—inch (203 mm) taper piles shall have a minimum embedment length of 6 feet (1828 mm) and shall project not more than 7 feet (2134 mm) above undisturbed ground surface.

AL105.4 Girders. Floor girders shall consist of solid sawn timber, built-up 2-inch- thick (51 mm) lumber, or trusses. Splices shall occur over wood piles. The floor girders shall span in the direction parallel to the potential floodwater and wave action.

AL105.5 Connections. Wood piles may be notched to provide a shelf for supporting the floor girders. The total notching shall not exceed 50 percent of the pile cross section. Approved bolted connections with ¼-inch (6.4 mm) corrosion-resistant or noncorrosive steel plates and ¾inch-diameter (19 mm) bolts shall be provided. Each end of the girder

